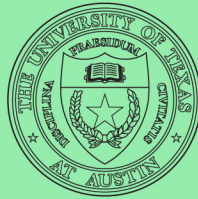


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## ENGINEERING PROPERTIES OF LAND RESOURCE UNITS IN THE CORPUS CHRISTI AREA

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## INTRODUCTION

Land resource units that are defined by their physical properties can be characterized quantitatively using engineering test data. Information about substrate properties in quantitative terms confirms and augments qualitative statements about the physical characteristics of land resource units. This information should allow a more accurate definition of environmental limits and may reduce the need for preliminary engineering investigations commonly undertaken to assess project feasibility and to estimate costs. Quantitative information should also aid planners in charting utilization of land consistent with its natural suitability or in identifying the approximate level of engineering necessary to render a particular site suitable, thereby maximizing resource use.

Consulting engineering firms, soils<sup>1</sup> testing firms, and many public agencies involved in construction have extensive files of site-specific soils test data necessary to properly assess foundation conditions.

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<sup>1</sup>The term "soil" or "soils" is used in this paper in an engineering sense. "The materials that constitute the earth's crust are rather arbitrarily divided by the civil engineer into two categories, soil and rock. Soil is a natural aggregate of mineral grains that can be separated by such gentle mechanical means as agitation in water" (Terzaghi and Peck, 1967, p. 4). As such, the term "soil" includes more than the material that is subject to the soil-forming processes in a pedological sense, and consequently includes unweathered sediments which are part of the geological substrate.

Rarely are test results extrapolated from previously tested areas because individual test values are considered valid only for the immediate test site.

By considering all soils test values within a given land resource unit, however, the unit can be characterized throughout its extent by a representative value or range of values. Such a characterization can be extended to untested but similar resource areas on a local or, perhaps with some limitations, on a regional scale. This characterization permits immediate maximum use of available data and is an alternative to an expensive, systematic testing program.

Land and water resources were mapped, and certain units were investigated quantitatively as part of a larger multidisciplinary study of the Corpus Christi area (fig. 1) including Nueces, San Patricio, Aransas, and Refugio Counties (Fruh and others, 1972, 1973a, and 1973b). Land and water resource units were defined according to criteria presented by St. Clair and others (1975) as modified from Brown and others (1971). Delineation of the resource units was based on mapping at a scale of 1:24,000 by Brown and others (1976) and McGowen and others (1976) for the Environmental Geologic Atlas of the Texas Coastal Zone. The map Land and Water Resources, Corpus Christi, Texas and the descriptions of the units presented by Kier and others (1974a) will be issued in the Bureau of Economic Geology's Land Resources Laboratory series (Kier and White, in preparation). Included in this report is a generalized land and water resources map of Nueces County (fig. 2).

Preliminary results of the pilot study to quantify land and water resource units in the Corpus Christi area were presented in Fisher and

others (1973) and in Kier and Bell (1974). The completed study, based on approximately twice as much data, was published in Kier and others (1974b). Because the availability of this report is limited, it has been modified and is being presented in this research note. Included are (1) a description of the basic data used, (2) the data management system employed, (3) the procedure used to analyze the data, (4) summaries and interpretation of the data, and (5) uses and interpretive limitations of the data.

#### BASIC DATA AND METHODOLOGY

More than 17,500 engineering test values were obtained from more than 7,000 core samples (fig. 3) representing nearly all the major, physically defined land resource units (table 1) which occur in the Corpus Christi area. Test data collected (table 2) are the types most commonly used by engineering testing laboratories to assess site suitability for commercial buildings, large single- and multiple-family developments, institutional facilities, and industrial plants. Descriptions of these tests are contained in appendix A.

Test values were recorded for each type of engineering test performed on samples from each previously mapped resource unit and grouped into 5-foot increments of depth extending to 50 feet below ground surface. Arithmetic means and standard deviations (one standard deviation) of engineering test values were then calculated for each depth interval. For example, in calculating a mean and standard deviation, the unit dry weight of a sample taken at a depth of 12 feet was combined with other

values of unit dry weight determined from samples from the same resource unit taken from a depth interval greater than 10 feet but less than or equal to 15 feet. Arithmetic means of test results characterize depth intervals within each resource unit by a single number. Standard deviations (normal distribution assumed) show probable ranges and variability of test results and, to some extent, allow prediction of future test values.

Numerous other kinds of statistical parameters are available, such as mode, median, mean deviation, and range, each with its own meaning and use. It was assumed that most readers would be familiar with the definitions of arithmetic mean and standard deviation, and that these statistical values were sufficient for the study. No adjustments were made for differences in the numbers of samples used in the calculations nor for skewness of data distribution. In future work, further statistical treatment of the engineering test values will undoubtedly provide additional information.

Assignment of test data to individual resource units was based on core locality in relation to land resource units at the surface, boring log descriptions, and prediction of sediment type at depth considering the interpreted environment of deposition. All core localities were plotted on 7.5- or 15-minute topographic maps, and the Universal Transverse Mercator (UTM) coordinates were determined to facilitate input into a computerized data storage bank. Specific data and core localities are confidential, however, to protect proprietary information.

Several land resource units with differing natural capability but

with similar physical properties were considered together in quantifying the engineering parameters. Most of these units could not be confidently distinguished at depth using normal core log descriptions. For example (see table 1), fore-island dunes and vegetated barrier flats (C2), and washover areas (C4) have similar physical properties; differences in natural suitability relate to other characteristics. Consequently, these units were lumped together into the general category of barrier islands (C), and the amount of test data for this resource unit was correspondingly increased.

The 5-foot class interval for depth calculations was chosen as a compromise between (1) the desire to show changes with depth, if any, in results of tests in individual capability units, and (2) the statistical validity of the mean and standard deviation calculations based on the anticipated amount of data to be collected for any one class interval--too few data in each class would lead to both fallacious means and fallacious standard deviations.

Fifty feet was chosen as the maximum average depth for confident prediction of the kinds of depositional environments in which the sediments accumulated. Furthermore, few data are available for soil samples deeper than 50 feet.

All calculations and construction of tables and graphs were performed by automatic data processing techniques (see appendix B for programs); all data are presently stored on magnetic tape. In plotting the graphs, mean values were arbitrarily assigned to the base of each 5-foot interval, and the curves were not smoothed.

## INTERPRETATION OF DATA

Tables 3 through 24 contain mean and standard deviation values of the 22 types of engineering tests for which sufficient data were available to justify performing calculations. Over 8,800 statistical calculations were made for the ten 5-foot depth intervals in each of the 20 land resource units. Trends and differences within and among resource units are evident when tabulated mean values are compared. For visual comparison, mean values of the engineering tests are displayed graphically in figures 4 through 17; for quick reference, mean values have been retabulated for the 0- to 5-foot depth interval in table 25. Characteristic ranges of means are shown in table 26.

Lands that are dominantly mud, for example low permeability mud (A-8), have low unit dry weights (96.1 to 100.8 lb/ft<sup>3</sup>, table 3, fig. 4), high natural moisture contents (23.9 to 28.1 percent, table 4, fig. 5), high shrink-swell potentials (linear drying shrinkage of 15.9 to 24.2 percent, table 14, fig. 13; plasticity indices of 30.8 to 42.6 percent, table 17, fig. 16), and low foundation strengths (standard penetrometer test values of 10 to 33 blows/ft, table 8, fig. 10).

Lands that are dominantly sand and silt, for example, moderately permeable sand and silt (A-3), have higher unit dry weights (99.0 to 108.6 lb/ft<sup>3</sup>, table 3, fig. 4), lower natural moisture contents (16.6 to 26.6 percent, table 4, fig. 5), lower shrink-swell potentials (linear drying shrinkage of approximately 7 percent, table 14, fig. 13; plasticity indices of 9.0 to 31.5 percent, table 17, fig. 16), and higher foundation strengths (standard penetrometer values of 17 blows/ft to 44

blows/ft, table 6, fig. 6; unconfined compression values of 1.7 to 2.4 tons/ft<sup>2</sup>, table 8, fig. 10).

Loose, highly permeable sand (A1) has even lower amounts of silt and clay than A3 (7.1 to 12.6 percent passing 200 mesh sieve, table 22, fig. 17). Unit dry weights of the loose sand vary from 94.6 to 107.2 lb/ft<sup>3</sup>\* at depth (table 3, fig. 4). Standard penetrometer values are approximately 7 blows/ft at the surface increasing to approximately 46 blows/ft at depth (table 6, fig. 6). The high range in mean values of unit dry weight results from an insufficient number of test values to make valid calculations or from compaction with increasing overburden (Krynine and Judd, 1957). Because the unit dry weights increase with depth, compaction of the sand is probably the overriding factor.

Sand-veneered low to moderate permeability sandy mud (A6) is composed of approximately 50 percent sand and 50 percent silt and clay (about 54 percent passing 200 sieve, table 22, fig. 17). The sandy mud is characterized by the highest unit dry weights of all the land resource units in the Corpus Christi area (107.7 to 110.1 lb/ft<sup>3</sup>, table 3, fig. 4), by natural moisture contents which are low near the surface but increase with depth (15.4 percent at the surface to 30.5 percent at depth, table 4, fig. 5), and by moderate to high foundation strengths (standard penetrometer values of 18 to 36 blows/ft, increasing with depth, table 6, fig. 6; unconfined compression values of 3.1 to 4.5 tons/ft<sup>2</sup>, table 8, fig. 10). The potential for shrink-swell is moderate

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\*Calculated from fewer than 10 test values.

(plasticity indices of 19.9 to 22.8 percent, table 17, fig. 16). High unit dry weights and high foundation strengths are to be expected in materials that are about 50 percent silt and clay and thus well graded (Krynine and Judd, 1957). In well-graded soils, fines occupy spaces between the coarser particles that would otherwise be void.

In the Corpus Christi area, made land and subaerial spoil (E) generally has a thickness not exceeding 20 feet; no test values were reported from spoil thicker than 20 feet. The material is characterized by low unit dry weights (94.8 to 102.2 lb/ft<sup>3</sup>, table 3, fig. 4), high natural moisture contents (21.5 to 27.0 percent, table 4, fig. 5), low foundation strengths (standard penetrometer values of 8 to 11 blows/ft, table 6, fig. 6; unconfined compression values of 0.8 to 1.2 tons/ft<sup>2</sup>, table 8, fig. 10) and high shrink-swell characteristics (plasticity indices of 23 to 31 percent,\* table 17, fig. 16).

Data obtained for other land resource units are few, and means and standard deviations calculated from these data are highly variable. Some tentative conclusions can be drawn, however. Barrier island resource units (C) are almost entirely sand (table 19, fig. 17). Foundation strengths on the islands are low at the surface, but increase with depth (standard penetrometer values of 20 blows/ft at the surface to 56 blows/ft at depth, table 6, fig. 6). Physical characteristics of barrier island sand are similar to physical characteristics of loose, highly permeable sand (A1) and engineering parameters should also be similar.

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\*Calculated from fewer than 10 test values.



Bay-bottom sediments (F) have high natural moisture contents (37.9 to 53.7 percent,\* table 4, fig. 5) as is expected, low shearing strengths (unconfined compression values of 0.1 to 0.5 tons/ft<sup>2</sup>, table 8, fig. 10) and high plasticity indices (20.3 to 34.8 percent, table 17, fig. 16). Conversely, bay-margin sands (F11) have lower natural moisture contents (values of 14.0 to 30.0 percent, table 4, fig. 5) because of the higher shell and sand content, but they also have low foundation strengths (standard penetrometer values of 11 to 16 blows/ft, table 6, fig. 6).

Graphs of the mean test values plotted against depth below surface elevation demonstrate several trends and anomalies. In general, foundation strength, as indicated by standard penetrometer values (fig. 6) and by Texas Highway Department cone penetrometer values (fig. 7), increases with depth. This is particularly true of material which has a high sand content (units A1, A3, C). Because the penetrometer indicates shearing strength, which for sands is dependent upon the amount of overburden, the blows per foot in a uniform sandy material should increase with depth (Krynine and Judd, 1957).

The statistical means of the unconfined and the triaxial compression tests (figs. 10, 11, and 12) show no definite trends. Samples of similar material subjected to the triaxial test generally do show increases in strength with increases in lateral confining pressure (Krynine and Judd, 1957), but this is not apparent from data we collected. There are several explanations for the absence of a trend: (1) the

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\*Calculated from fewer than 10 test values.

number of samples tested was too small to give valid means and standard deviations; (2) unconfined compression strength values were recorded whether or not failure occurred on slickensided surfaces; and (3) soils engineers generally consider triaxial tests on materials from the Corpus Christi area unreliable because of the difficulty involved in obtaining undisturbed samples and returning them to the laboratory for testing (Joseph Stapp, 1973, personal communication).

Mean hand penetrometer values (fig. 8) also vary widely with depth. This variation is probably due to inaccuracy of the original measurement rather than to lack of test values (see definition in appendix A).

#### LIMITATIONS OF QUANTITATIVE CHARACTERIZATION

Inherent sources of variability in quantitative characterization of land resource units include (1) imprecise logging and sample description by the testing laboratory, (2) errors in assigning test data to the proper land resource units, (3) differences in methods used by the various agencies and firms and differences in their precision in performing essentially the same kind of test, and (4) lack of sufficient data for valid statistical calculations. Errors in logging or recognizing the proper land resource unit, and differences in test precision among and within the various testing firms can be overcome only by using a sufficient number of test values to statistically eliminate fallacious data. There is no way to compensate for lack of data.

Furthermore, it must be understood that mean values presented for engineering parameters of land resource units are only averages and may

not represent actual values at any given locality. The values are a planning tool and should not be construed as a substitute for actual site data; onsite testing will still be required to properly assess foundation requirements. In addition, test results that are outside the limits of one standard deviation will be obtained. Such readings can result from end-member compositions within a unit or scale factor where a land resource substrate penetrated by test core is too small to be shown at the scale at which land resource units are mapped.

#### Utility and Application

Despite its limitations, quantitative information regarding substrate parameters, such as the data presented in this research, provides basic input for assessing engineering characteristics of mapped land resource units and for estimating the less easily quantified substrate parameters of land resources. Substrate parameters which are difficult to quantify include (1) foundation strength, (2) plasticity, (3) compressibility, (4) slope stability, (5) shrink-swell potential, (6) ease of excavation, (7) fill potential, (8) relative rate of internal drainage, (9) moisture-retention capacity, (10) waste-disposal capability, (11) ability to handle septic tank effluent, (12) corrosion potential, and (13) general suitability for construction purposes.

Such quantitative and nonquantitative data allow a more accurate definition of an area's natural environmental suitability than can be provided by qualitative maps alone. Information provided by the data may reduce the need for preliminary engineering investigations commonly undertaken to assess project feasibility and to estimate costs, and may

allow consideration of more sites than is normally possible in preliminary studies. Quantitative information should aid planners in charting land utilization that maximizes resource use without significantly impairing environmental quality.

#### SUMMARY--QUANTITATIVE ENGINEERING CHARACTERIZATION

A method of quantifying engineering parameters of natural land resource units in the Corpus Christi area has been developed combining engineering test data available from public agencies and private firms with analysis and maps of land and water resources. Statistical means and standard deviations of values from 21 different kinds of engineering tests have been calculated in many resource units as a function of depth. Although there are limitations in the procedure, results from the calculations correlate well with expected characteristics of the natural units. Quantitative knowledge of substrate characteristics should aid planners in charting optimum use of land resources, particularly when placed in a comprehensive environmental data package providing a wide range of natural resource information.

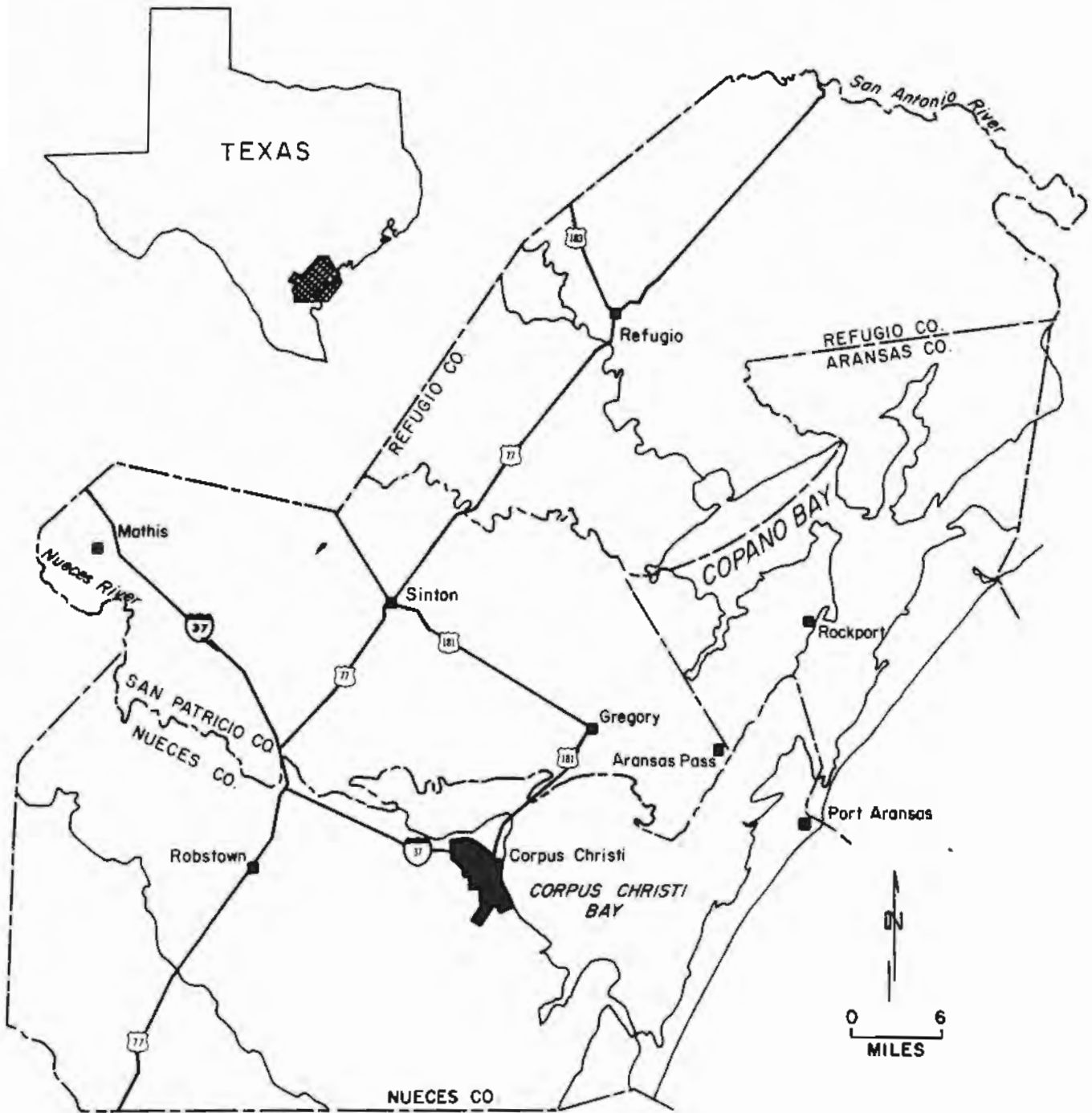


Figure 1. Index map to Corpus Christi area--Nueces, San Patricio, Aransas, and Refugio Counties, Texas.

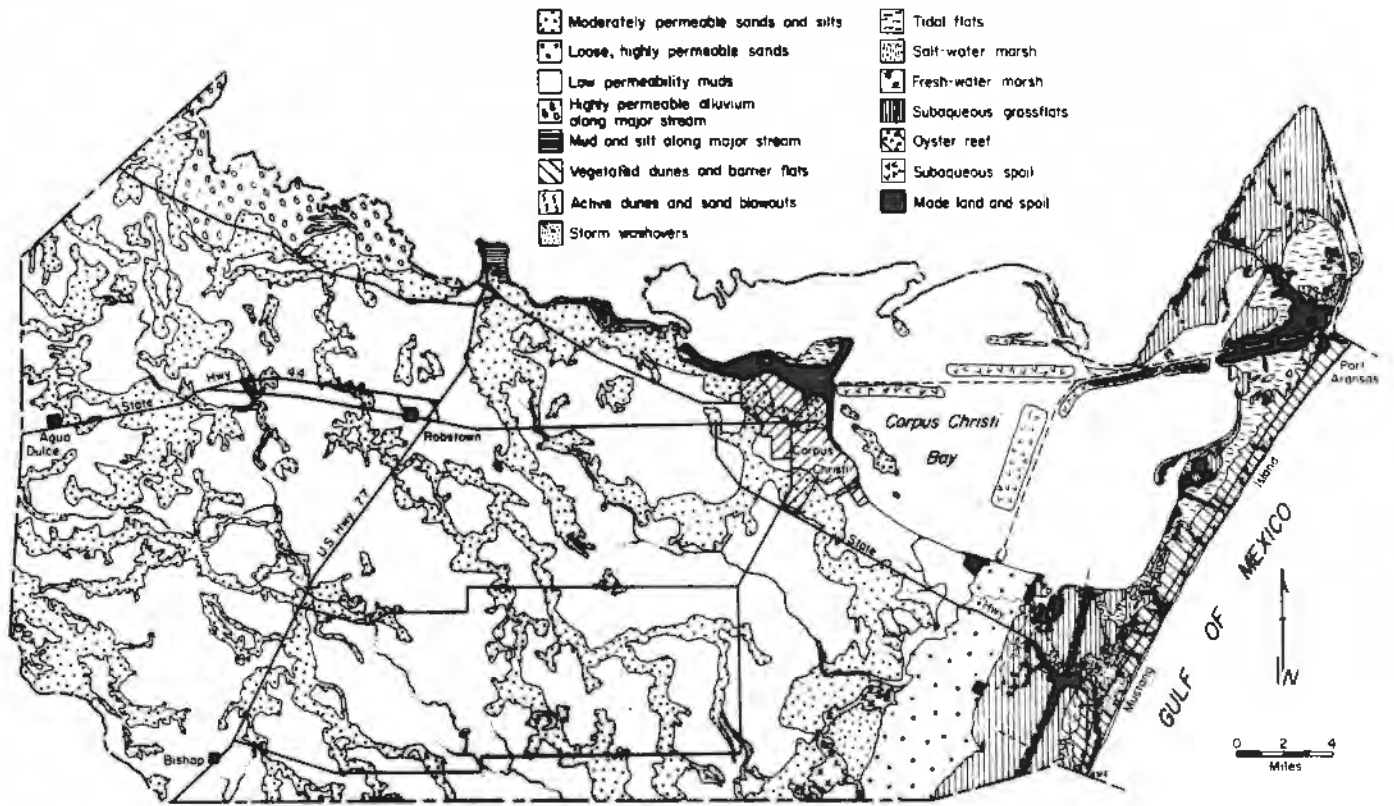


Figure 2. Generalized land and water resources map, Nueces County, Texas.

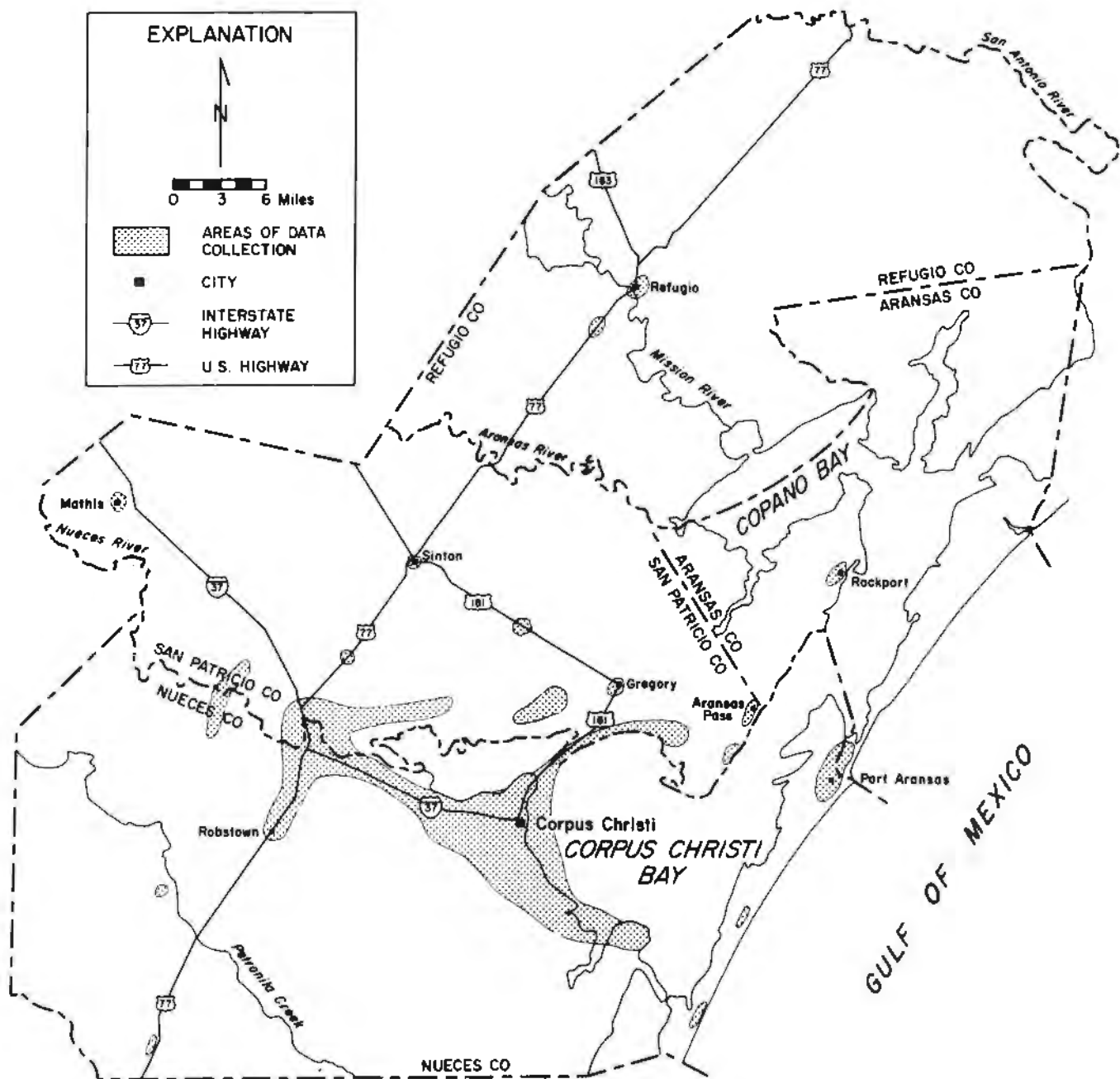


Figure 3. Distribution of engineering test data from the Corpus Christi area.

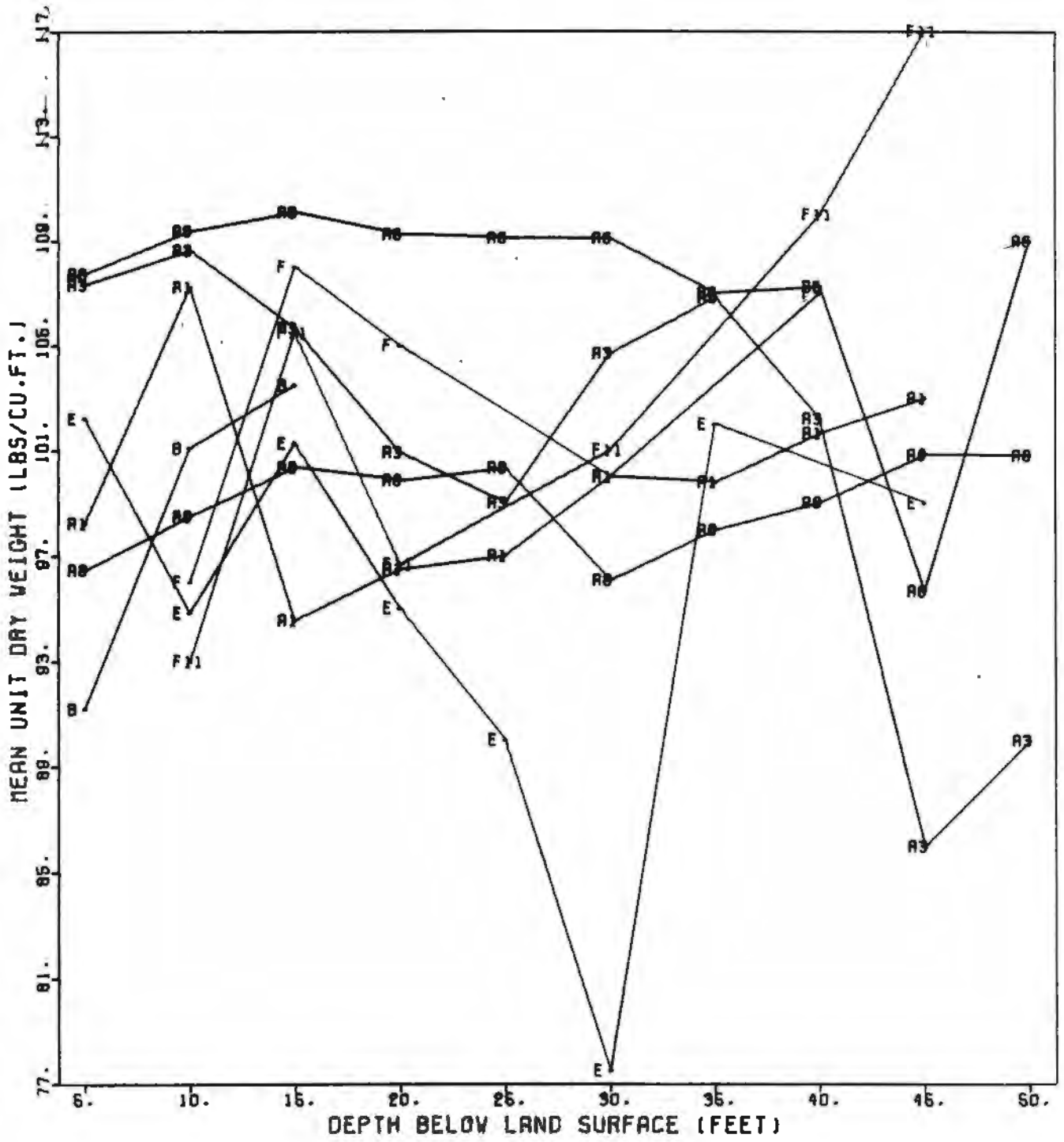


Figure 4. Curves showing variation of mean unit dry weight with depth.



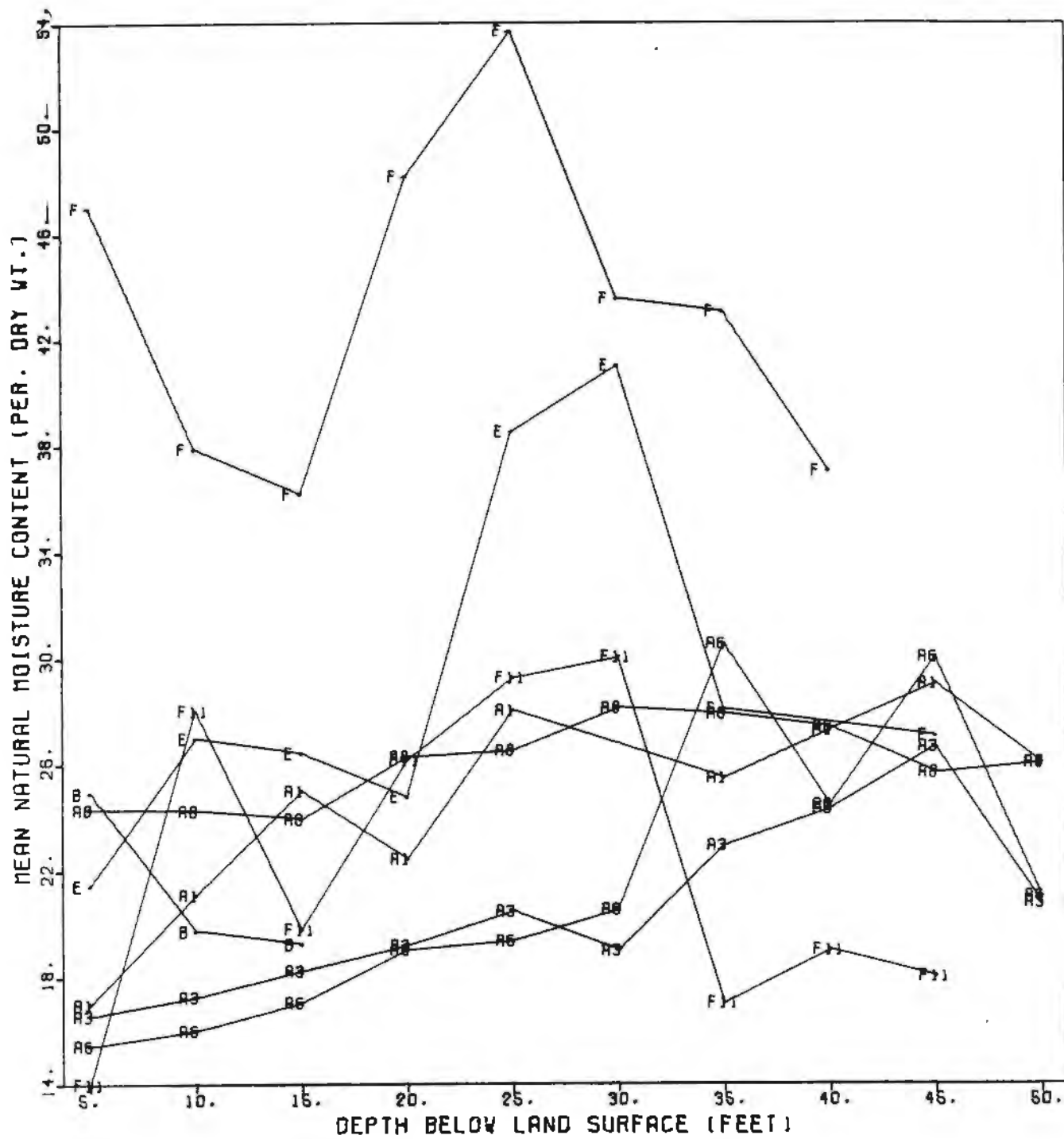


Figure 5. Curves showing variation of mean natural moisture content with depth.

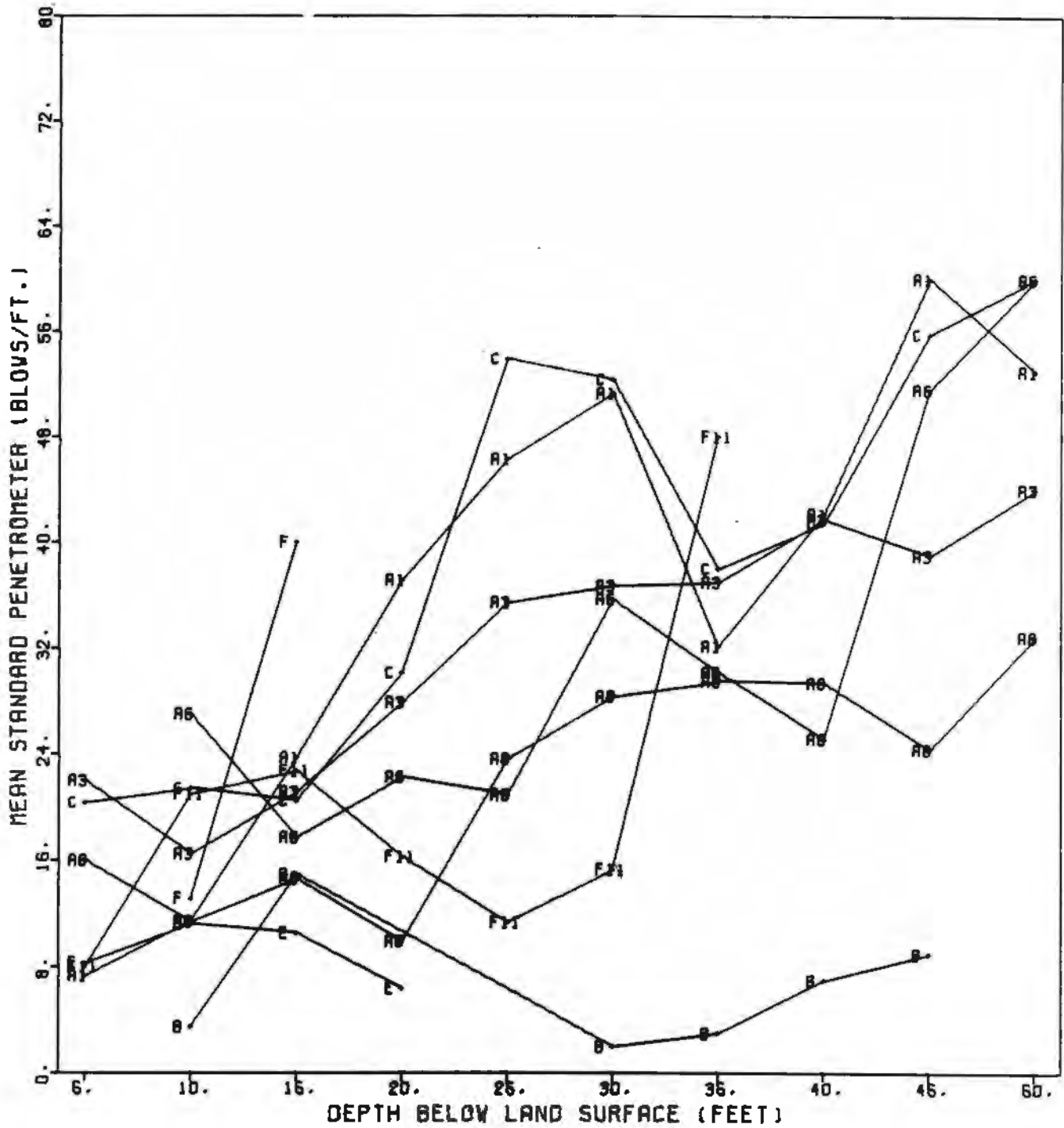


Figure 6. Curves showing variation of mean standard penetrometer values with depth.

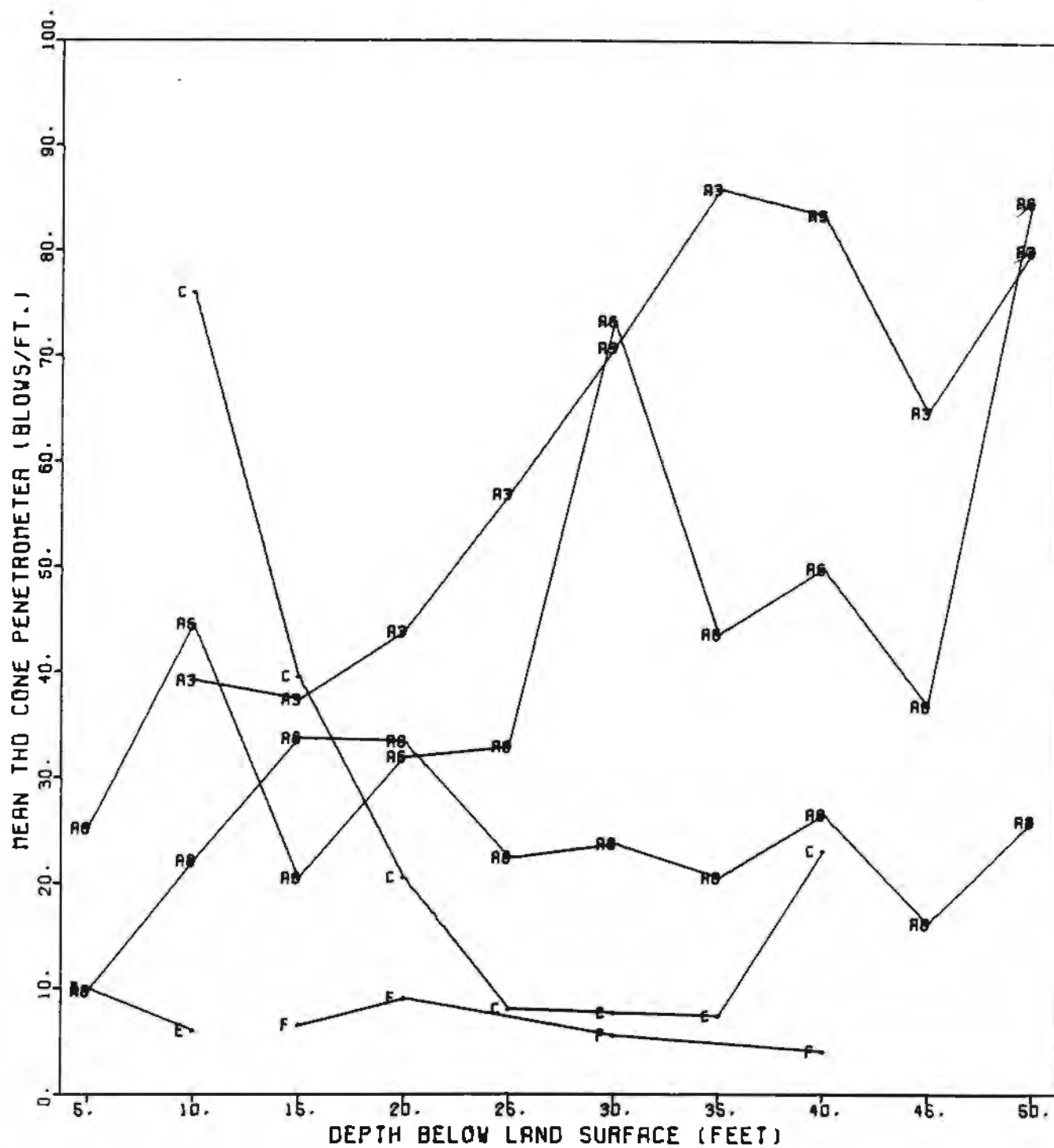


Figure 7. Curves showing variation of mean Texas Highway Department penetrometer values with depth.

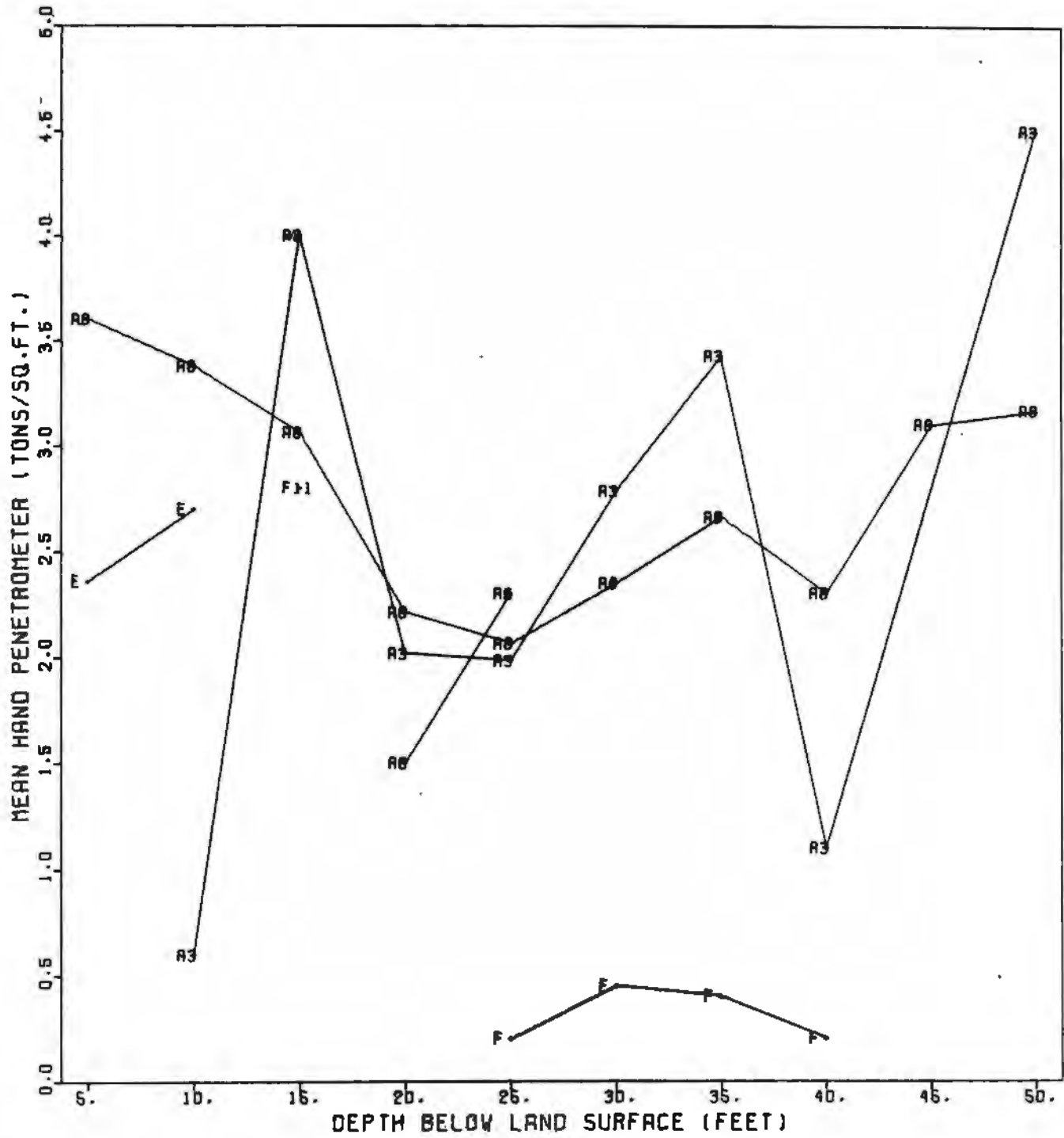


Figure 8. Curves showing variation of mean hand penetrometer values with depth.

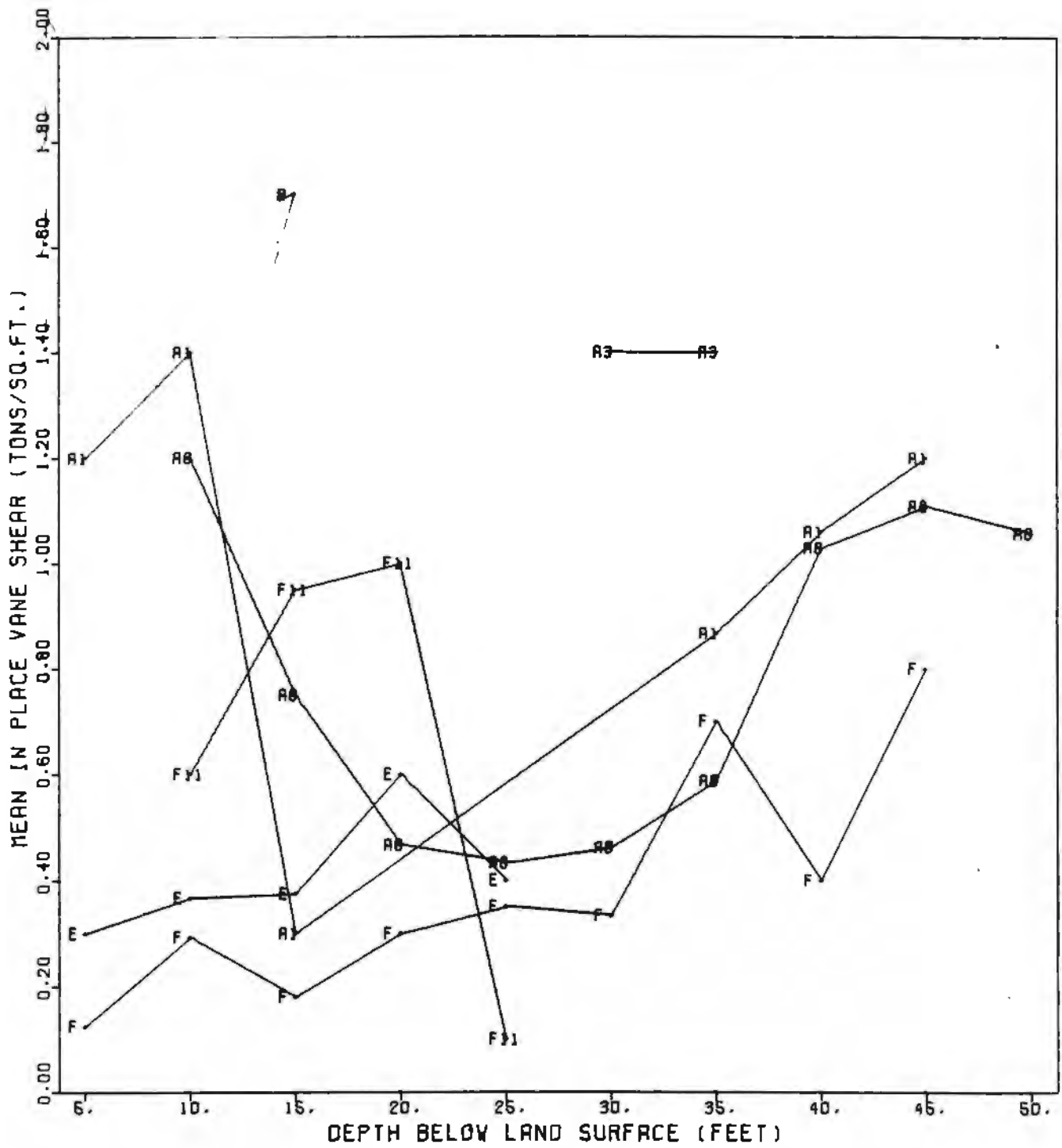


Figure 9. Curves showing variation of mean in-place vane shear values with depth.

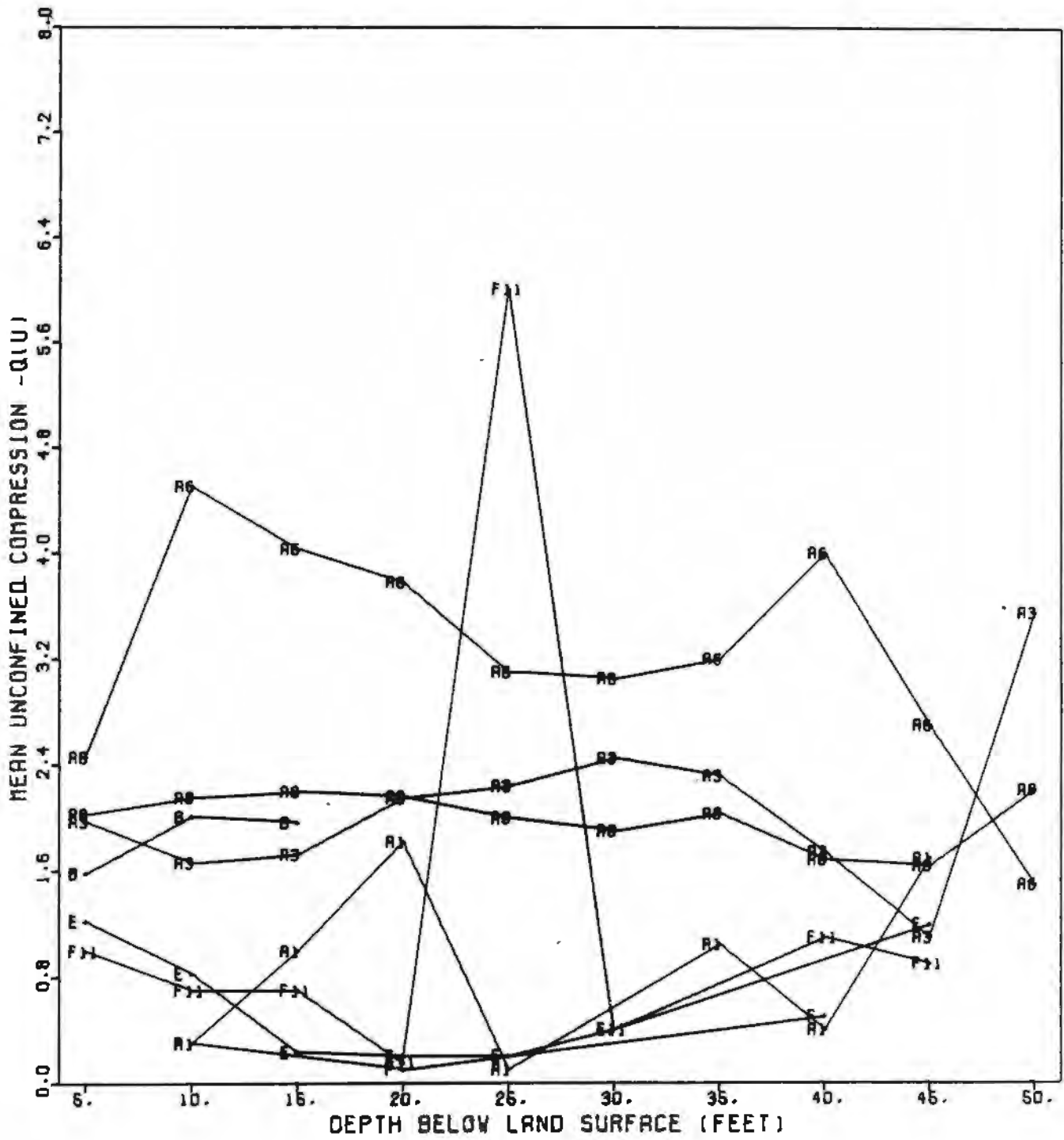


Figure 10. Curves showing variation of mean unconfined compression test values ( $Q_u$ ) with depth.

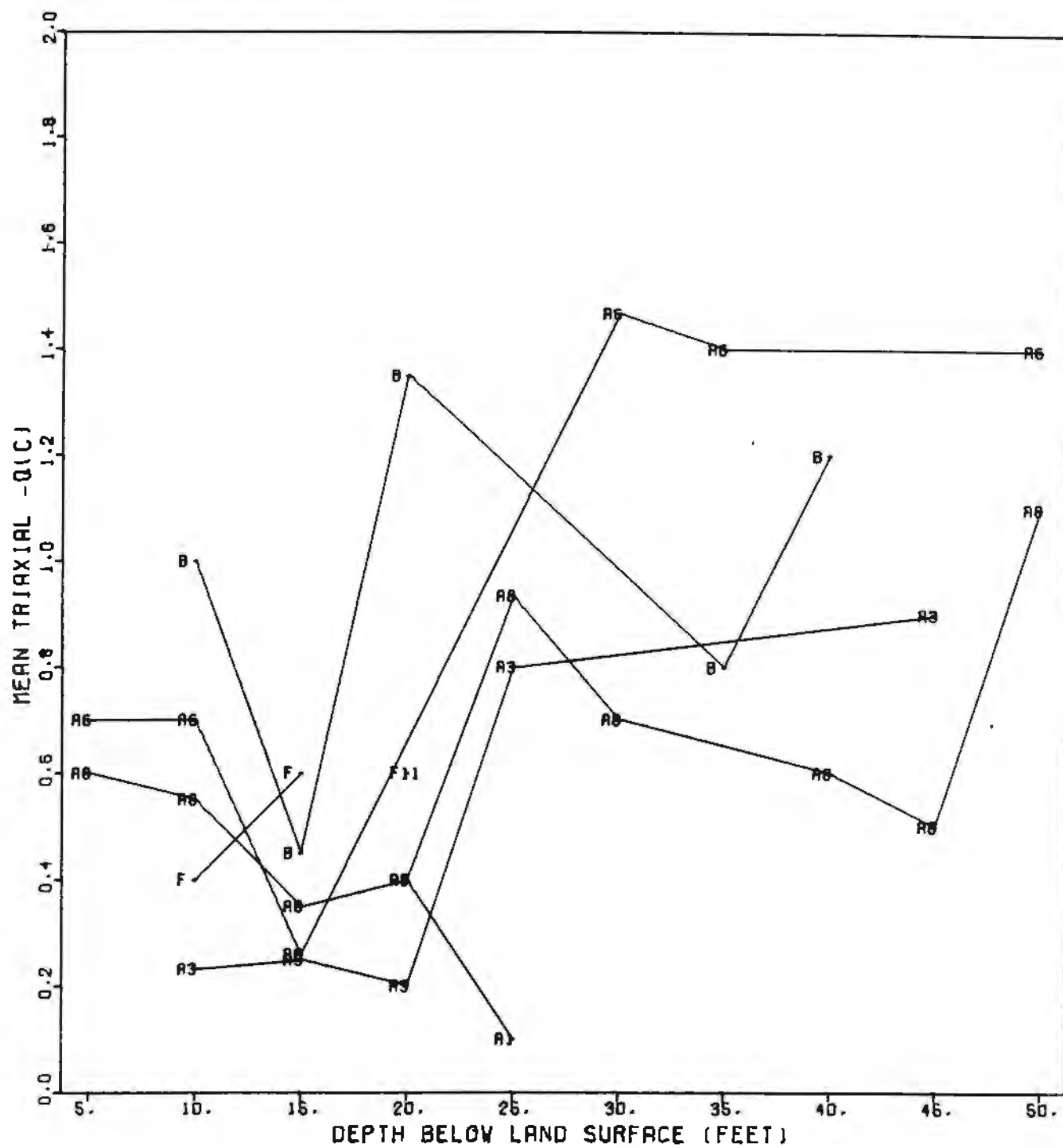


Figure 11. Curves showing variation of mean triaxial test values ( $Q_c$ ) with depth.

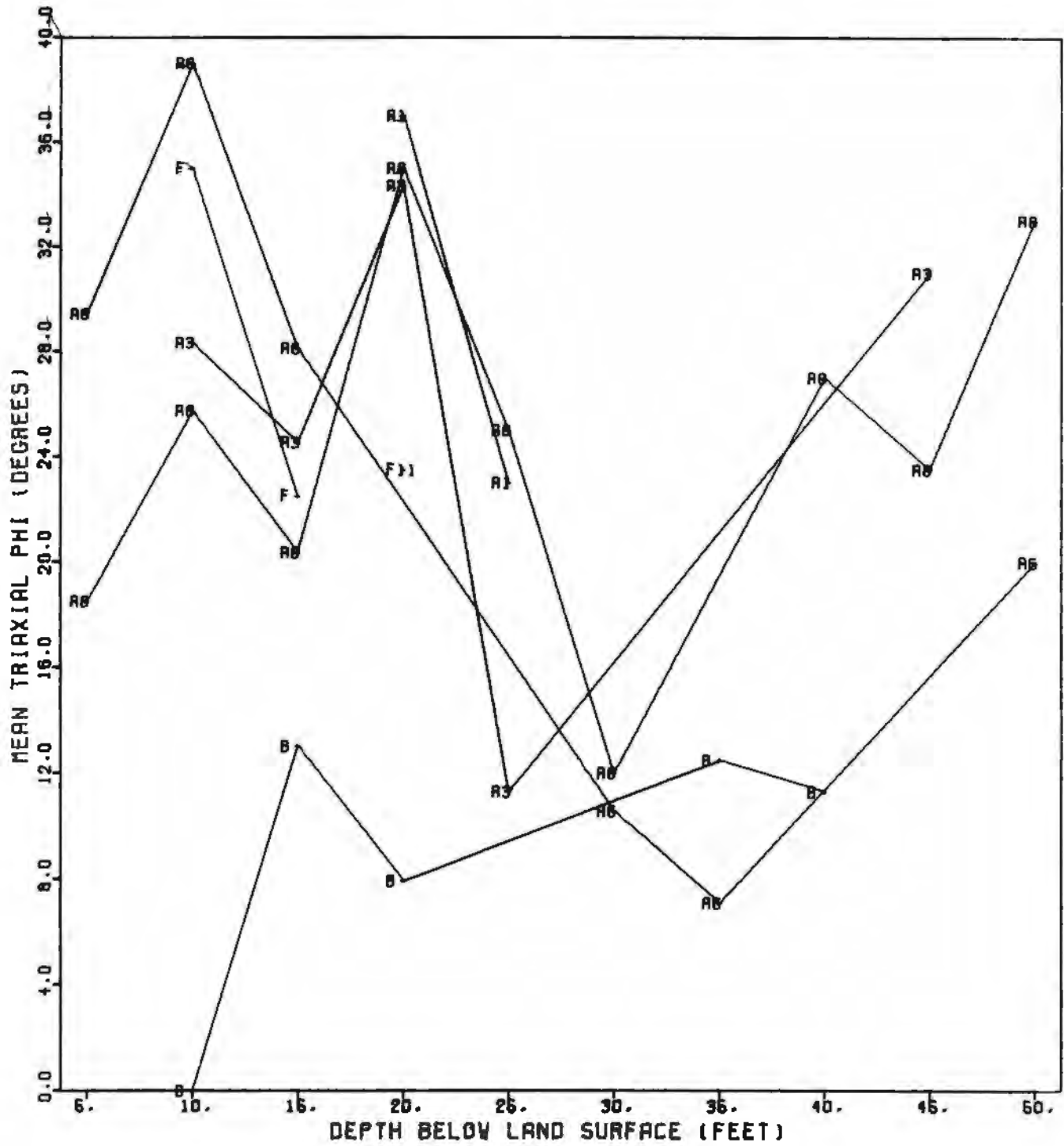


Figure 12. Curves showing variation of mean triaxial phi values ( $\phi$ ) with depth.



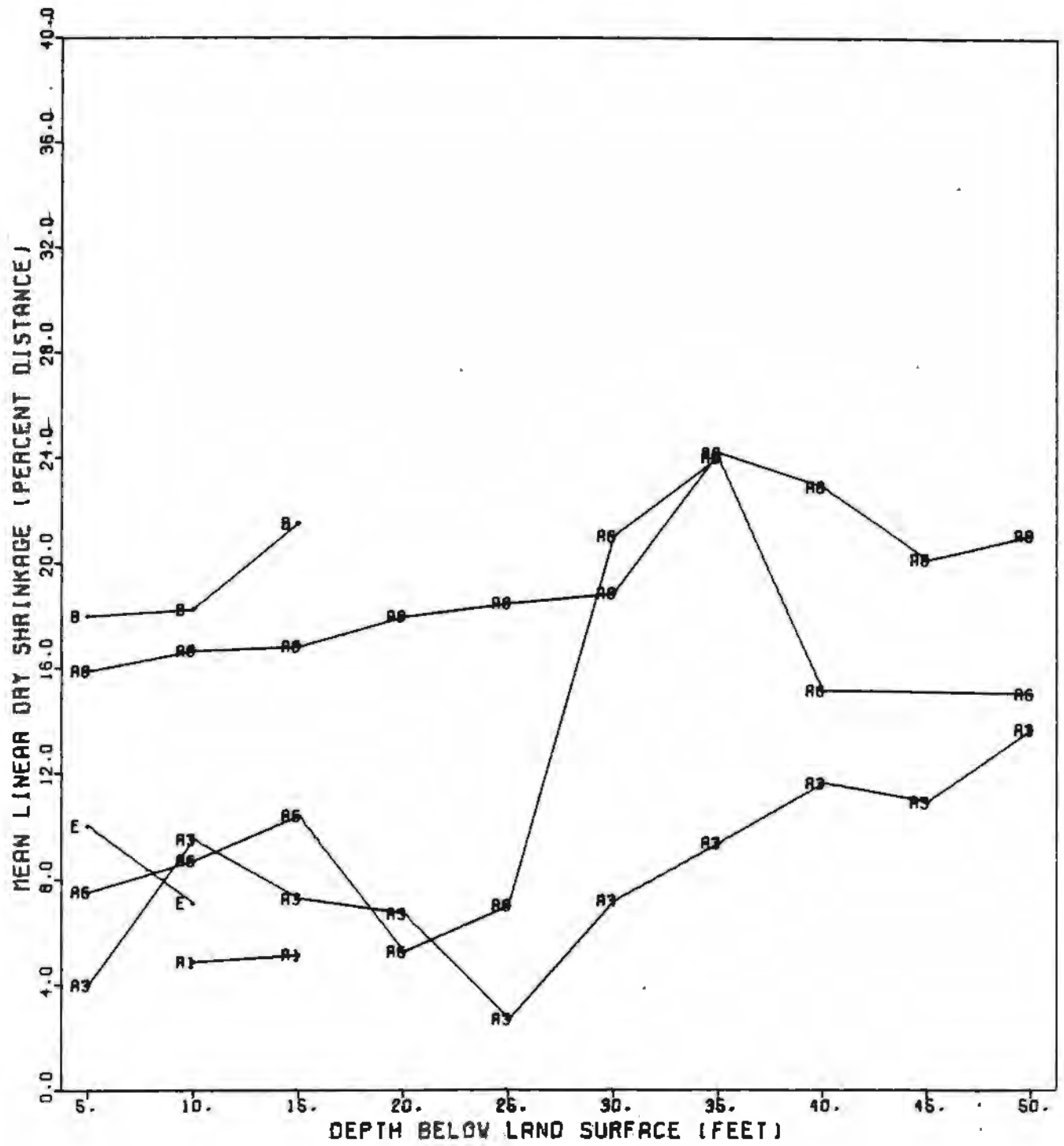


Figure 13. Curves showing variation of mean linear drying shrinkage with depth.

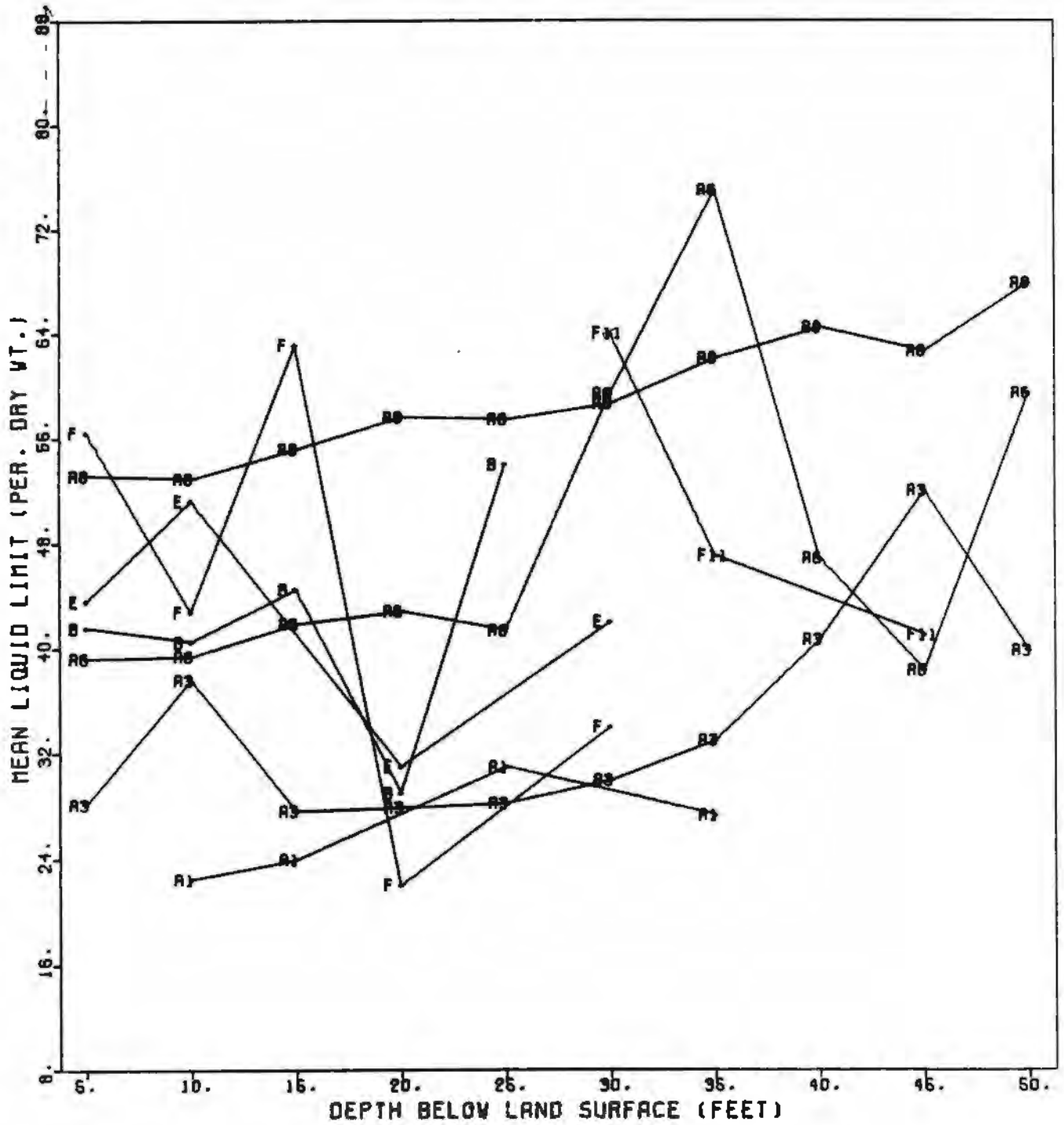


Figure 14. Curves showing variation of mean liquid limit values with depth.

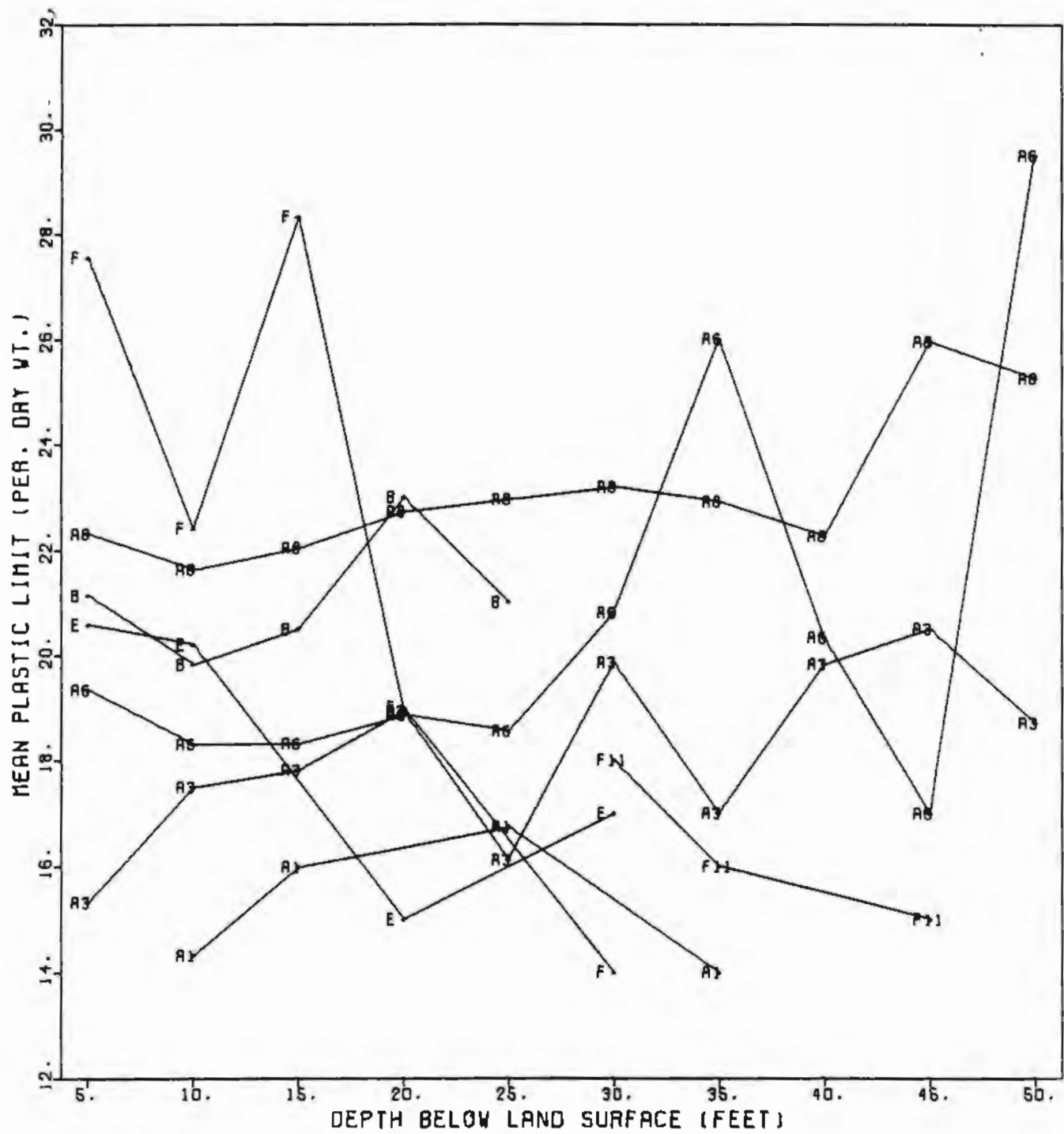


Figure 15. Curves showing variation of mean plastic limit values with depth.

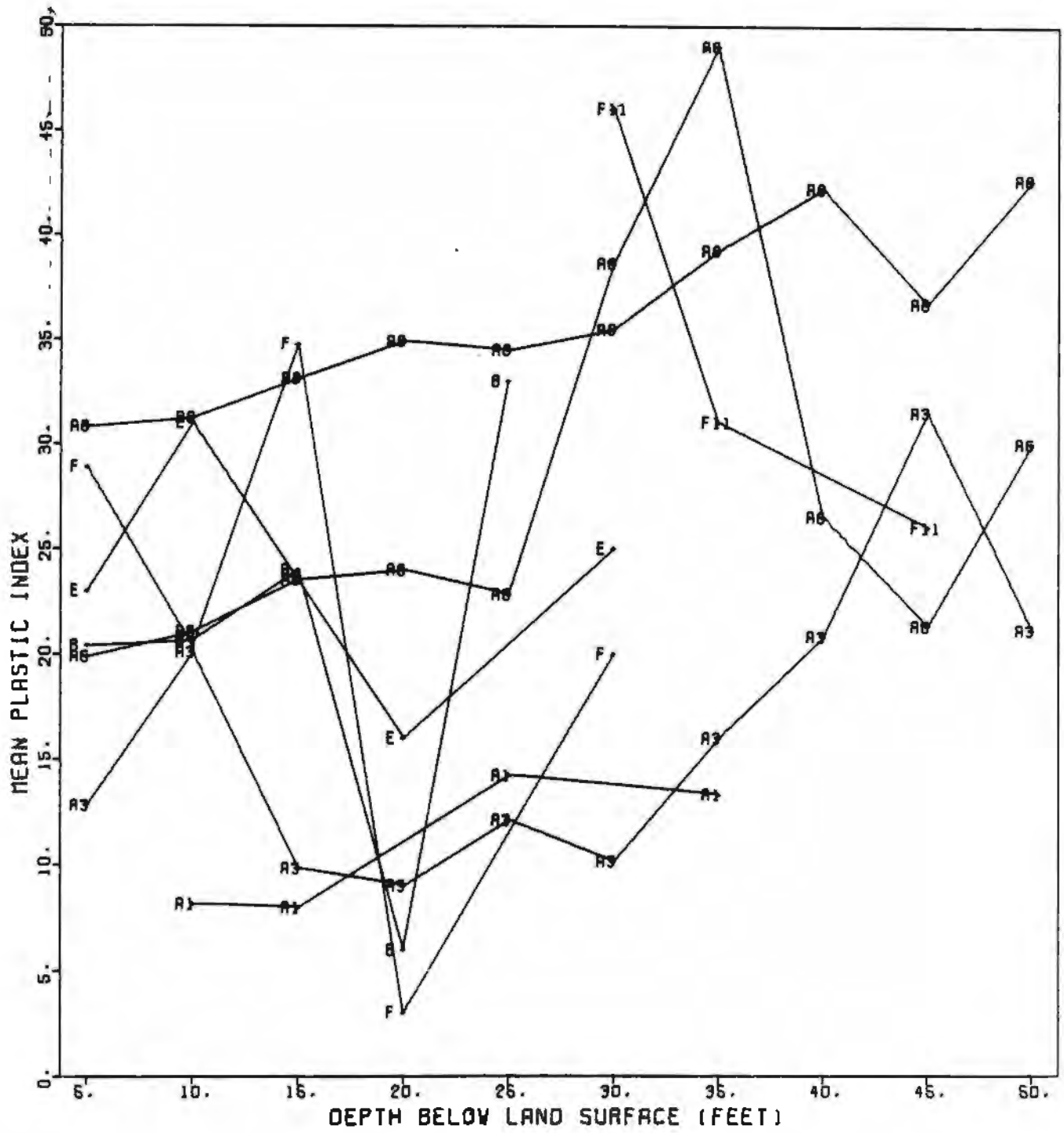


Figure 16. Curves showing variation of mean plasticity index values with depth.

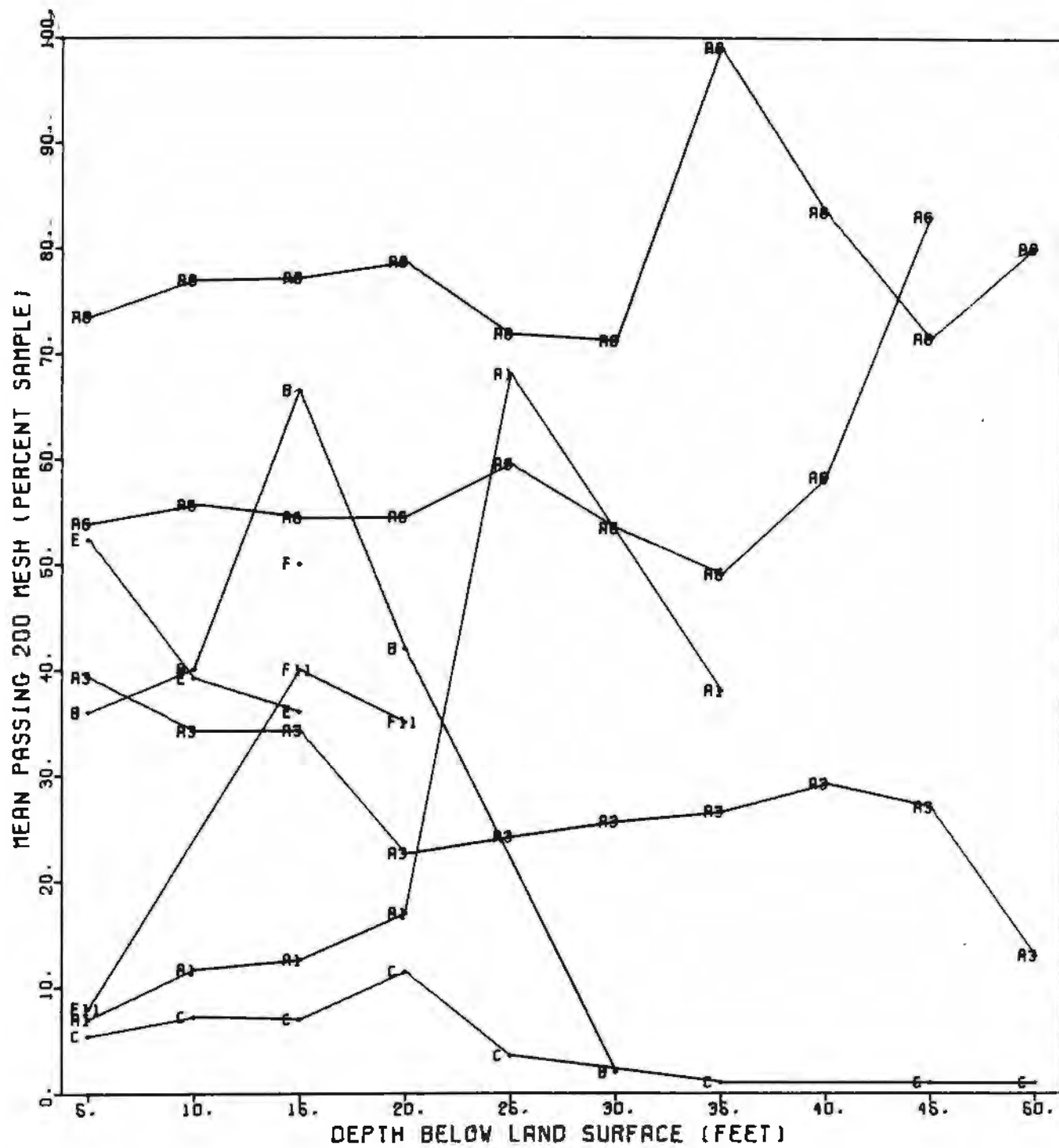


Figure 17. Curves showing variation with depth of mean values for percentage of sample passing 200 mesh sieve.

Table 1. Land resource units—Corpus Christi area.\*

## COASTAL PLAIN

A1	Sand, recharge, highly permeable: thin, shallow aquifer, maximum thickness approximately 50 feet, high water table, low moisture-retention capacity, poor waste-disposal capability, excavation easy, low shrink-swell potential, high foundation strength, very high corrosion potential, locally tree-covered
A3	Sand and silt, moderately permeable: shallow aquifer, thickness up to 100 feet, low to moderate water-retention capacity, excavation easy, low shrink-swell potential, high foundation strength, high corrosion potential, moderately susceptible to erosion, mostly cropland
A6	Sandy mud, low to moderate permeability, moderately permeable sand veneer: veneer highly variable in thickness, moderate water-retention capacity, moderate to good waste-disposal capability, excavation moderate, moderate to high shrink-swell potential, low to moderate foundation strength, very high corrosion potential, mixed range and cropland
A8	Mud, low permeability: moderate to high water-retention capacity, cracks extensively when dry, good waste-disposal capability, excavation difficult to moderate, high shrink-swell potential, low foundation strength, very high corrosion potential, commonly cropland
A11	Sand, calichified: low to moderate permeability, moderate to low water-retention capacity, poor to moderate waste-disposal capability, excavation moderate, low shrink-swell potential, high foundation strength, low corrosion potential, commonly rangeland

## ACTIVE FLOOD PLAINS

B1	Sand and gravel, highly permeable: point bar deposits, highly susceptible to flooding, high water table, poor waste-disposal capability, excavation easy, low shrink-swell potential, generally high foundation strength, high to very high corrosion potential, locally tree-covered, some unmapped mud lenses
B2	Mud and silt, low to moderate permeability: overbank deposits, high susceptibility to flooding, high water table, generally occurs in topographic lows that pond water easily, poor waste-disposal capability, excavation difficult to moderate, high shrink-swell potential, low foundation strength, very high corrosion potential, locally good cropland
B3	Natural levee, elevated: mixed mud and sand, susceptible to flooding during extreme floods, essential for flood protection during lesser floods, poor waste-disposal capability, excavation moderate, variable shrink-swell potential, low to moderate foundation strength, very high corrosion potential, locally tree-covered
B4	Alluvium, small active streams: sand, silt, and mud, highly susceptible to flooding and bank erosion, poor waste-disposal capability, low foundation strength and bank stability, locally tree-covered

## BARRIER ISLANDS

C2	Fore-island dunes and vegetation-stabilized barrier flats: sand and shell, highly permeable, perched brackish- to fresh-water aquifer, highly susceptible to flooding and erosion by storm tides, salt-tolerant grasses dominant vegetation, maintenance of vegetation critical in preventing wind and water erosion and providing natural barrier to storm surge, very poor waste-disposal capability, very high corrosion potential, poor to fair construction site, detrimental sand excavation
C4	Storm washover areas, washover channels and fans: loose sand and shell, subject to extensive flooding by storm tides with scour and fill and significant transport of sediment during hurricanes and other storms subject to extensive modification by wind between major floods, very poor waste-disposal capability, very high corrosion potential, extremely poor construction site
C5	Tidal flats: mixed mud, sand, and shell, subject to rapid and sudden inundation by astronomical, storm, and wind-driven tides, moderate wind erosion between floods, poor waste-disposal capability, very high corrosion potential, poor construction site

## MAN-MADE FEATURES

E1	Made land and subaerial spoil: mixed mud, silt, sand, and shell, composition and physical properties highly variable, locally steep relief, locally subject to extensive erosion, poor solid waste-disposal capability, very high corrosion potential, construction should be undertaken with caution
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## BAYS, LAGOONS, ESTUARIES, AND OPEN GULF

F1-5	Bay and estuarine bottom material: mostly mud and sandy mud, local concentrations of sand and shell, high organic content, high water content, overlying water mass varies from restricted or enclosed to tidally influenced, brackish to normal marine salinities, poor waste-disposal capability, low foundation strength
F11	Local shell and sand beaches and berms: thin deposits of sand and shell, commonly flooded and modified by storms, highly susceptible to erosion, poor waste-disposal capability, poor construction site

\*Units described in this table are only those for which quantitative data are reported here. Units were not renumbered for this report. B1, B2, B3, and B4 equal B in the tables and graphs; C2, C4, and C5 equal C in the tables and graphs; E1 equals E in the tables and graphs; and F1-5 equals F in the tables and graphs. For more complete descriptions of the units, see Kier and White, in preparation.

Table 2. Physical test data.

1. Unit dry weight ( $\text{lb}/\text{ft}^3$ )
2. Natural moisture content (percentage of dry weight)
3. Foundation strength
  - a. Standard penetrometer (blows/foot)
  - b. Texas Highway Department cone penetrometer (blows/foot)
  - c. Unconfined compressive strength ( $\text{tons}/\text{ft}^2$ )
  - d. Triaxial shear strength ( $\text{tons}/\text{ft}^2$  or  $\phi$ )
  - e. Hand penetrometer ( $\text{tons}/\text{ft}^2$ )
  - f. In-place vane shear ( $\text{tons}/\text{ft}^2$ )
4. Linear drying shrinkage (percentage of distance)
5. Atterburg limits
  - a. Liquid limit (percentage of dry weight)
  - b. Plastic limit (percentage of dry weight)
  - c. Plasticity index
6. Grain size distribution--passing 200 mesh (percentage of sample)
7. Void ratio (volume of voids/volume of solids)

Table 3. Mean unit dry weight (pounds per cubic foot).  
Land Resource Units

DEPTH		A1	A3	A6	AB	R	E	F	F11
0-5	MN	98.2	107.3	107.7	96.5	91.2	102.2		
	SD	1.1	7.4	7.1	7.9	6.7	7.4		
	NO	4	50	104	336	15	38		
5-10	MN	107.2	108.6	109.4	98.5	101.1	94.8	96.0	93.0
	SD	2.9	6.2	7.1	7.2	6.4	7.3	0.0	0.0
	NO	4	59	166	418	11	24	1	1
10-15	MN	94.6	105.7	110.1	100.4	103.5	101.3	108.0	105.5
	SD	6.7	6.2	5.6	7.5	2.1	13.4	2.0	9.5
	NO	9	63	141	326	4	7	2	2
15-20	MN	96.5	100.9	109.2	99.9		95.0	105.0	96.7
	SD	3.1	19.8	6.1	8.0		1.0	13.0	5.7
	NO	6	55	106	218		2	2	3
20-25	MN	97.0	99.0	109.1	100.3		90.0		
	SD	4.2	17.7	5.7	8.5		0.0		
	NO	6	27	51	154		1		
25-30	MN	100.0	104.7	109.1	96.1		77.5	100.0	101.0
	SD	5.0	4.1	4.4	14.9		5.5	0.0	0.0
	NO	2	17	12	103		2	1	1
30-35	MN	99.8	106.8	107.0	98.0		102.0		
	SD	4.8	6.6	3.2	10.0		0.0		
	NO	5	6	4	88		1		
35-40	MN	101.7	102.2	107.2	99.0			107.0	110.0
	SD	8.0	9.5	3.5	11.0			9.0	0.0
	NO	3	5	4	65			2	1
40-45	MN	103.0	86.0	95.7	100.8		99.0		117.0
	SD	0.0	24.8	20.3	9.7		0.0		0.0
	NO	1	3	3	58		1		1
45-50	MN		90.0	109.0	100.8				
	SD		26.0	7.0	7.9				
	NO		2	2	43				



Table 4. Mean natural moisture content (percent dry weight).

## Land Resource Units

DEPTH		A1	A3	A6	A8	B	E	F	F11
0-5	MN	17.0	16.6	15.4	24.3	24.9	21.5	47.0	14.0
	SD	4.2	5.6	5.0	6.1	7.5	6.2	7.6	0.0
	NO	8	49	105	362	15	66	6	1
5-10	MN	21.1	17.3	16.0	24.3	19.7	27.0	37.9	28.0
	SD	4.6	6.5	5.5	5.7	6.1	10.5	12.4	0.0
	NO	10	74	170	452	12	45	15	1
10-15	MN	25.0	18.2	17.0	23.9	19.2	26.4	36.2	19.8
	SD	5.7	6.6	7.0	7.3	1.1	8.2	13.9	6.0
	NO	8	111	149	356	4	24	6	5
15-20	MN	22.4	19.2	19.0	26.3		24.7	48.2	26.1
	SD	3.7	6.5	3.9	11.2		1.8	24.0	6.4
	NO	9	123	111	248		4	6	8
20-25	MN	28.0	20.5	19.4	26.5		38.5	53.7	29.2
	SD	5.0	7.0	5.2	11.3		7.5	10.1	13.9
	NO	5	67	55	189		2	3	9
25-30	MN		19.0	20.5	28.1		41.0	43.5	30.0
	SD		6.7	4.9	11.4		8.6	14.5	3.6
	NO		46	17	129		3	4	3
30-35	MN	25.4	22.9	30.5	27.8		28.0	43.0	17.0
	SD	2.5	4.3	11.9	9.5		0.0	13.0	0.0
	NO	9	12	10	119		1	3	1
35-40	MN	27.2	24.3	24.4	27.3			37.0	19.0
	SD	7.4	5.5	10.5	8.0			20.7	0.0
	NO	4	13	9	94			3	1
40-45	MN	29.0	26.6	30.0	25.6		27.0		18.0
	SD	5.0	16.4	15.1	8.4		0.0		0.0
	NO	2	13	4	82		1		1
45-50	MN	26.0	20.7	21.0	26.0				
	SD	0.0	4.1	4.1	6.3				
	NO	1	16	4	68				

Table 5. Mean in-place vane shear (tons per square foot).

		Land Resource Units						
DEPTH		A1	A3	A8	B	E	F	F11
0-5	MN	1.2				.3	.1	
	SD	.2				0.0	.0	
	NO	2				2	4	
5-10	MN	1.4		1.2		.4	.3	.6
	SD	0.0		.2		.1	.2	0.0
	NO	1		2		3	15	1
10-15	MN	.3		.7	1.7	.4	.2	.9
	SD	0.0		.7	0.0	.2	.1	.2
	NO	1		2	1	4	10	2
15-20	MN			.5		.6	.3	1.0
	SD			.5		0.0	.1	0.0
	NO			3		1	2	1
20-25	MN			.4		.4	.4	.1
	SD			.5		0.0	.3	0.0
	NO			12		1	4	1
25-30	MN		1.4	.5			.3	
	SD		0.0	.3			.3	
	NO		1	13			3	
30-35	MN	.9	1.4	.6			.7	
	SD	.5	0.0	.4			0.0	
	NO	3	1	16			1	
35-40	MN	1.1		1.0			.4	
	SD	.2		.4			0.0	
	NO	5		17			1	
40-45	MN	1.2		1.1			.8	
	SD	0.0		.3			.4	
	NO	1		12			2	
45-50	MN			1.1				
	SD			.5				
	NO			7				

Table 6. Mean standard penetrometer (blows per foot).

## Land Resource Units

DEPTH		A1	A3	A6	A8	B	C	E	F	F11
0-5	MN	7.3	22.0		16.0		20.4	8.3		8.0
	SD	4.2	19.3		0.0		8.0	7.5		0.0
	NO	31	5		1		14	21		1
5-10	MN	11.5	16.5	27.0	11.4	3.5	21.4	11.3	13.1	21.0
	SD	4.8	14.6	0.0	8.0	1.5	6.3	12.3	7.1	7.7
	NO	39	32	1	10	2	16	20	7	4
10-15	MN	23.8	21.2	17.7	14.6	15.0	20.5	10.6	40.0	22.8
	SD	15.3	14.7	14.0	12.1	0.0	6.6	11.4	0.0	15.5
	NO	33	125	11	20	1	16	18	1	9
15-20	MN	37.2	27.9	22.3	9.9		30.1	6.4		16.3
	SD	16.2	16.4	16.4	5.7		12.3	5.9		13.0
	NO	24	224	15	23		14	8		20
20-25	MN	46.3	35.5	20.9	23.7		53.9			11.4
	SD	17.0	20.2	14.8	14.5		8.3			12.0
	NO	9	155	12	29		13			17
25-30	MN	51.3	36.7	35.7	28.3	2.0	52.3			15.3
	SD	5.7	17.8	16.5	19.3	0.0	7.8			8.1
	NO	7	97	18	26	1	12			3
30-35	MN	32.1	37.0	30.2	29.5	3.0	38.0			48.0
	SD	24.2	17.6	24.4	17.2	0.0	11.0			0.0
	NO	7	60	6	26	1	12			1
35-40	MN	42.2	41.7	25.2	29.4	7.0	41.5			
	SD	14.9	16.2	21.0	18.4	0.0	10.9			
	NO	6	57	6	14	1	11			
40-45	MN	60.0	39.0	51.6	24.4	9.0	55.8			
	SD	0.0	16.7	11.2	15.8	0.0	6.9			
	NO	2	53	5	11	1	11			
45-50	MN	53.0	44.1	60.0	32.9		60.0			
	SD	7.0	41.0	0.0	19.0		0.0			
	NO	2	34	1	11		2			

Table 7. Mean Texas Highway Department cone penetrometer (blows per foot).

## Land Resource Units

DEPTH		A3	A6	AB	C	E	F
0-5	MN		25.2	9.8		10.0	
	SD		14.8	5.5		0.0	
	NO		6	4		1	
5-10	MN	39.2	44.5	22.1	76.0	6.0	
	SD	27.2	26.8	7.4	0.0	0.0	
	NO	6	8	9	1	1	
10-15	MN	37.3	20.4	33.7	39.5		6.5
	SD	24.0	16.4	29.7	1.5		.5
	NO	17	9	6	2		2
15-20	MN	43.7	31.9	33.3	20.5		9.0
	SD	32.5	30.6	27.9	2.5		0.0
	NO	11	9	12	2		1
20-25	MN	56.8	32.9	22.4	8.0		
	SD	40.0	38.6	10.7	.8		
	NO	23	8	11	3		
25-30	MN	70.7	73.2	23.6	7.7		5.5
	SD	38.4	39.6	12.8	3.7		2.5
	NO	13	4	14	3		2
30-35	MN	85.8	43.5	20.5	7.3		
	SD	29.9	43.6	22.5	.5		
	NO	9	8	17	3		
35-40	MN	83.4	49.7	26.4	23.0		4.0
	SD	27.2	40.6	27.9	0.0		0.0
	NO	8	8	15	1		1
40-45	MN	64.7	36.7	16.1			
	SD	30.0	38.1	15.7			
	NO	12	7	16			
45-50	MN	80.1	84.7	25.8			
	SD	28.9	19.3	25.4			
	NO	10	4	11			

Table 8. Mean unconfined compression ( $Q_u$ ) (tons per square foot).

## Land Resource Units

DEPTH		A1	A3	A6	A8	B	E	F	F11
0-5	MN		2.0	2.5	2.0	1.6	1.2		1.0
	SD		1.4	.9	1.2	.6	.6		0.0
	NO		9	5	156	13	27		1
5-10	MN	.3	1.7	4.5	2.2	2.0	.8	.3	.7
	SD	0.0	1.1	2.9	1.0	.5	.6	0.0	0.0
	NO	1	36	48	229	9	15	2	1
10-15	MN	1.0	1.7	4.0	2.2	2.0	.2		.7
	SD	0.0	1.1	1.8	1.3	.5	.0		.5
	NO	1	72	48	205	3	3		2
15-20	MN	1.8	2.2	3.8	2.2		.2	.1	.1
	SD	.5	1.0	1.4	1.3		0.0	0.0	.1
	NO	4	72	31	135		1	1	2
20-25	MN	.1	2.2	3.1	2.0		.2		6.0
	SD	0.0	1.0	1.1	1.0		0.0		0.0
	NO	1	40	22	85		1		1
25-30	MN		2.4	3.0	1.9		.4		.4
	SD		.9	.7	1.4		0.0		0.0
	NO		30	2	74		1		1
30-35	MN	1.0	2.3	3.2	2.0				
	SD	.8	.9	0.0	1.4				
	NO	2	6	1	61				
35-40	MN	.4	1.7	4.0	1.7			.5	1.1
	SD	0.0	1.2	0.0	1.7			.4	0.0
	NO	1	7	1	43			2	1
40-45	MN	1.7	1.1	2.7	1.6		1.2		.9
	SD	0.0	1.0	0.0	1.7		0.0		0.0
	NO	1	5	1	46		1		1
45-50	MN		3.5	1.5	2.2				
	SD		1.3	0.0	1.9				
	NO		6	1	28				

Table 9. Mean triaxial shear ( $Q_c$ ) (tons per square foot).  
Land Resource Units

DEPTH		A1	A3	A6	A8	B	F	F11
0-5	MN			.7	.6			
	SD			.4	.4			
	NO			3	2			
5-10	MN		.2	.7	.5	1.0	.4	
	SD		.2	0.0	.1	0.0	0.0	
	NO		3	1	4	1	1	
10-15	MN		.3	.3	.3	.4	.6	
	SD		.0	.1	.1	.1	0.0	
	NO		2	5	2	2	1	
15-20	MN	.4	.2		.4	1.3		.6
	SD	0.0	.1		0.0	.1		0.0
	NO	1	3		1	2		1
20-25	MN	.1	.8		.9			
	SD	0.0	0.0		.5			
	NO	1	1		3			
25-30	MN			1.5	.7			
	SD			.8	.2			
	NO			3	2			
30-35	MN			1.4		.8		
	SD			0.0		0.0		
	NO			1		1		
35-40	MN				.6	1.2		
	SD				0.0	0.0		
	NO				1	1		
40-45	MN		.9		.5			
	SD		0.0		.1			
	NO		1		2			
45-50	MN			1.4	1.1			
	SD			0.0	0.0			
	NO			1	1			

Table 10. Mean triaxial phi (degrees).  
Land Resource Units

DEPTH		A1	A3	A6	A8	B	F	F11
0-5	MN			29.4	18.5			
	SD			.6	10.5			
	NO			3	2			
5-10	MN		28.3	39.0	25.7	.0	35.0	
	SD		3.4	0.0	14.7	0.0	0.0	
	NO		3	1	4	1	1	
10-15	MN		24.5	28.1	20.4	13.1	22.5	
	SD		3.6	1.3	3.3	11.1	0.0	
	NO		2	5	2	2	1	
15-20	MN	37.0	34.3		35.0	7.9		23.5
	SD	0.0	3.0		0.0	4.5		0.0
	NO	1	3		1	2		1
20-25	MN	23.0	11.3		25.0			
	SD	0.0	0.0		1.4			
	NO	1	1		3			
25-30	MN			10.6	12.0			
	SD			11.2	3.0			
	NO			3	2			
30-35	MN			7.1		12.5		
	SD			0.0		0.0		
	NO			1		1		
35-40	MN				27.0	11.3		
	SD				0.0	0.0		
	NO				1	1		
40-45	MN		31.0		23.5			
	SD		0.0		5.5			
	NO		1		2			
45-50	MN			20.0	33.0			
	SD			0.0	0.0			
	NO			1	1			

Table 11. Mean hand penetrometer (tons per square foot).  
Land Resource Units

DEPTH		A3	A6	A8	E	F	F11
0-5	MN			3.6	2.4		
	SD			1.0	.8		
	NO			63	5		
5-10	MN	.6		3.4	2.7		
	SD	.1		1.3	0.0		
	NO	3		72	1		
10-15	MN	4.0		3.1			2.8
	SD	0.0		1.4			0.0
	NO	1		53			1
15-20	MN	2.0	1.5	2.2			
	SD	1.3	0.0	1.5			
	NO	6	1	41			
20-25	MN	2.0	2.3	2.1		.2	
	SD	1.1	0.0	1.7		0.0	
	NO	6	1	35		1	
25-30	MN	2.8		2.3		.4	
	SD	1.5		1.6		.4	
	NO	6		29		2	
30-35	MN	3.4		2.7		.4	
	SD	1.3		1.7		.3	
	NO	4		34		2	
35-40	MN	1.1		2.3		.2	
	SD	.1		1.7		0.0	
	NO	2		9		1	
40-45	MN			3.1			
	SD			1.3			
	NO			5			
45-50	MN	4.5		3.2			
	SD	0.0		1.4			
	NO	1		3			



Table 12. Mean absorption swell (percent volume).  
Land Resource Units

DEPTH		A3	A6	A8	B
0-5	MN		3.9	4.0	3.5
	SD		2.1	3.4	0.0
	NO		8	27	1
5-10	MN		2.0	3.5	
	SD		1.5	2.3	
	NO		6	12	
10-15	MN	1.2	1.5	2.5	
	SD	0.0	.4	1.1	
	NO	1	2	3	
15-20	MN			5.0	
	SD			0.0	
	NO			1	
20-25	MN				
	SD				
	NO				
25-30	MN				
	SD				
	NO				
30-35	MN				
	SD				
	NO				
35-40	MN				
	SD				
	NO				
40-45	MN				
	SD				
	NO				
45-50	MN				
	SD				
	NO				

Table 13. Mean absorption pressure (tons per square foot).  
Land Resource Units

DEPTH		A3	A6	A8	B
0-5	MN	.5	2.6	3.2	1.2
	SD	0.0	2.0	3.9	0.0
	NO	1	9	27	1
5-10	MN		1.4	1.2	
	SD		1.2	.9	
	NO		6	12	
10-15	MN	.3	.4	1.2	
	SD	0.0	.2	.5	
	NO	1	3	3	
15-20	MN			1.3	
	SD			.3	
	NO			2	
20-25	MN			.7	
	SD			0.0	
	NO			1	
25-30	MN				
	SD				
	NO				
30-35	MN				
	SD				
	NO				
35-40	MN				
	SD				
	NO				
40-45	MN				
	SD				
	NO				
45-50	MN				
	SD				
	NO				

Table 14. Mean linear drying shrinkage (percent distance).  
Land Resource Units

DEPTH		A1	A3	A6	A8	B	E
0-5	MN		4.0	7.5	15.9	18.0	10.0
	SD		2.3	3.3	3.5	1.9	3.1
	NO		3	5	46	2	11
5-10	MN	4.9	9.5	8.7	16.7	18.2	7.1
	SD	.5	3.8	3.8	4.0	1.8	2.1
	NO	4	4	8	46	2	2
10-15	MN	5.1	7.3	10.4	16.8	21.5	
	SD	.5	3.6	3.6	5.7	0.0	
	NO	2	14	5	29	1	
15-20	MN		6.7	5.3	18.0		
	SD		4.2	4.8	6.3		
	NO		12	2	21		
20-25	MN		2.7	7.0	18.5		
	SD		1.6	0.0	7.2		
	NO		5	1	20		
25-30	MN		7.2	21.0	18.8		
	SD		5.5	6.1	7.8		
	NO		4	4	13		
30-35	MN		9.4	24.0	24.2		
	SD		4.6	0.0	3.5		
	NO		3	1	7		
35-40	MN		11.6	15.1	22.9		
	SD		6.7	5.4	5.3		
	NO		4	4	6		
40-45	MN		10.9		20.1		
	SD		9.7		8.9		
	NO		8		7		
45-50	MN		13.7	15.0	21.0		
	SD		7.6	7.0	6.8		
	NO		6	2	6		

Table 15. Mean liquid limit (percent dry weight).  
Land Resource Units

DEPTH		A1	A3	A6	A8	B	E	F	F11
0-5	MN		28.2	39.2	53.1	41.6	43.6	56.4	
	SD		6.7	6.5	10.6	20.0	11.9	14.7	
	NO		6	20	94	7	19	9	
5-10	MN	22.5	37.6	39.4	52.9	40.5	51.2	42.7	
	SD	3.6	17.1	7.3	12.3	16.3	22.0	10.8	
	NO	6	10	26	86	6	5	15	
10-15	MN	24.0	27.6	41.9	55.2	44.5		63.1	
	SD	3.0	5.7	14.1	16.0	23.5		11.3	
	NO	2	22	22	70	2		9	
15-20	MN		27.9	42.9	57.6	29.0	31.0	22.0	
	SD		9.4	11.5	16.8	0.0	0.0	0.0	
	NO		16	7	47	1	1	1	
20-25	MN	31.0	28.2	41.4	57.4	54.0			
	SD	1.2	6.8	7.4	20.5	0.0			
	NO	4	6	11	44	1			
25-30	MN		30.0	59.4	58.6		42.0	34.0	64.0
	SD		9.8	12.2	23.2		0.0	0.0	0.0
	NO		7	5	38		1	1	1
30-35	MN	27.3	33.0	75.0	62.1				47.0
	SD	4.1	8.9	0.0	20.6				0.0
	NO	3	4	1	31				1
35-40	MN		40.7	46.8	64.4				
	SD		12.9	18.1	19.5				
	NO		6	6	20				
40-45	MN		52.0	38.3	62.6				41.0
	SD		29.1	7.6	20.1				0.0
	NO		10	3	22				1
45-50	MN		39.9	59.5	67.9				
	SD		18.1	15.5	18.0				
	NO		7	2	16				

Table 16. Mean plastic limit (percent dry weight).  
Land Resource Units

DEPTH		A1	A3	A6	A8	B	E	F	F11
0-5	MN		15.3	19.3	22.3	21.1	20.6	27.6	
	SD		1.6	2.6	4.8	5.6	3.4	4.3	
	NO		6	20	94	7	19	9	
5-10	MN	14.3	17.5	18.3	21.6	19.8	20.2	22.4	
	SD	2.7	3.1	2.3	5.0	4.4	7.2	5.0	
	NO	6	10	26	86	6	5	15	
10-15	MN	16.0	17.8	18.3	22.0	20.5		28.3	
	SD	2.0	2.2	5.0	5.1	5.5		2.6	
	NO	2	22	22	70	2		9	
15-20	MN		18.9	18.9	22.7	23.0	15.0	19.0	
	SD		3.6	3.2	5.1	0.0	0.0	0.0	
	NO		16	7	47	1	1	1	
20-25	MN	16.7	16.1	18.5	23.0	21.0			
	SD	.4	5.4	3.2	5.8	0.0			
	NO	4	8	11	44	1			
25-30	MN		19.9	20.8	23.2		17.0	14.0	18.0
	SD		2.9	2.6	8.1		0.0	0.0	0.0
	NO		7	5	38		1	1	1
30-35	MN	14.0	17.0	26.0	22.9				16.0
	SD	.8	3.5	0.0	6.5				0.0
	NO	3	4	1	31				1
35-40	MN		19.8	20.3	22.2				
	SD		5.6	8.3	6.0				
	NO		6	6	20				
40-45	MN		20.5	17.0	26.0				15.0
	SD		8.8	.8	10.0				0.0
	NO		10	3	22				1
45-50	MN		18.7	29.5	25.2				
	SD		5.6	1.5	6.2				
	NO		7	2	16				

Table 17. Mean plasticity index.  
Land Resource Units

DEPTH		A1	A3	A6	A8	B	E	F	F11
0-5	MN		12.8	19.9	30.8	20.4	23.0	28.9	
	SD		5.1	5.5	8.3	15.2	10.6	12.0	
	NO		6	20	94	7	19	9	
5-10	MN	8.2	20.1	21.1	31.2	20.7	31.0	20.3	
	SD	1.2	14.9	7.3	8.8	14.1	18.8	7.7	
	NO	6	10	26	86	6	5	15	
10-15	MN	8.0	9.8	23.5	33.1	24.0		34.8	
	SD	1.0	7.0	9.8	12.5	18.0		12.5	
	NO	2	22	22	70	2		9	
15-20	MN		9.0	24.0	34.9	6.0	16.0	3.0	
	SD		8.2	10.6	13.9	0.0	0.0	0.0	
	NO		16	7	47	1	1	1	
20-25	MN	14.2	12.1	22.8	34.5	33.0			
	SD	1.6	7.2	5.9	16.2	0.0			
	NO	4	8	11	44	1			
25-30	MN		10.1	38.6	35.4		25.0	20.0	46.0
	SD		9.9	13.7	17.4		0.0	0.0	0.0
	NO		7	5	38		1	1	1
30-35	MN	13.3	16.0	49.0	39.2				31.0
	SD	4.6	7.2	0.0	15.7				0.0
	NO	3	4	1	31				1
35-40	MN		20.8	26.5	42.2				
	SD		11.2	10.3	16.1				
	NO		6	6	20				
40-45	MN		31.5	21.3	36.7				26.0
	SD		22.8	7.5	14.1				0.0
	NO		10	3	22				1
45-50	MN		21.1	30.0	42.6				
	SD		13.2	14.0	13.7				
	NO		7	2	16				

Table 18. Mean percent gravel in sample.  
Land Resource Units

DEPTH		A1	A3	A6	A8	C	E	F11
0-5	MN	.0			.0	.0		
	SD	.0			0.0	0.0		
	NO	4			3	2		
5-10	MN	.0			.0	.0	1.0	
	SD	0.0			0.0	0.0	0.0	
	NO	1			3	2	1	
10-15	MN				3.0	.0		
	SD				5.2	0.0		
	NO				4	2		
15-20	MN		.0		1.0			.0
	SD		0.0		1.4			0.0
	NO		1		3			2
20-25	MN					.0		
	SD					0.0		
	NO					1		
25-30	MN		.0					
	SD		0.0					
	NO		2					
30-35	MN		.0			.0		
	SD		0.0			0.0		
	NO		1			2		
35-40	MN		6.0					
	SD		0.0					
	NO		1					
40-45	MN					.0		
	SD					0.0		
	NO					2		
45-50	MN					.0		
	SD					0.0		
	NO					1		

Table 19. Mean percent sand in sample.  
Land Resource Units

DEPTH		A1	A3	A6	AB	C	E	F11
0-5	MN	91.3			47.7	99.0		
	SD	1.1			11.9	0.0		
	NO	4			3	2		
5-10	MN	92.0			55.0	98.0	26.0	
	SD	0.0			5.4	0.0	0.0	
	NO	1			3	2	1	
10-15	MN				32.5	99.0		
	SD				13.4	0.0		
	NO				4	2		
15-20	MN		90.0		32.0			65.0
	SD		0.0		22.6			33.0
	NO		1		3			2
20-25	MN					98.0		
	SD					0.0		
	NO					1		
25-30	MN		75.5					
	SD		3.5					
	NO		2					
30-35	MN		77.0			99.0		
	SD		0.0			0.0		
	NO		1			2		
35-40	MN		77.0					
	SD		0.0					
	NO		1					
40-45	MN					99.0		
	SD					0.0		
	NO					2		
45-50	MN					99.0		
	SD					0.0		
	NO					1		



Table 20. Mean percent silt in sample.  
Land Resource Units

DEPTH		A3	AB	E
0-5	MN		19.3	
	SD		2.9	
	NO		3	
5-10	MN		13.7	28.0
	SD		.9	0.0
	NO		3	1
10-15	MN		20.2	
	SD		4.4	
	NO		4	
15-20	MN		29.7	
	SD		11.3	
	NO		3	
20-25	MN			
	SD			
	NO			
25-30	MN	12.0		
	SD	0.0		
	NO	1		
30-35	MN			
	SD			
	NO			
35-40	MN	13.0		
	SD	0.0		
	NO	1		
40-45	MN			
	SD			
	NO			
45-50	MN			
	SD			
	NO			

Table 21. Mean percent clay in sample.  
Land Resource Units

DEPTH		A3	AB	E
0-5	MN		33.3	
	SD		10.7	
	NO		3	
5-10	MN		31.3	45.0
	SD		4.6	0.0
	NO		3	1
10-15	MN		44.2	
	SD		9.7	
	NO		4	
15-20	MN		37.3	
	SD		13.0	
	NO		3	
20-25	MN			
	SD			
	NO			
25-30	MN	9.0		
	SD	0.0		
	NO	1		
30-35	MN			
	SD			
	NO			
35-40	MN	4.0		
	SD	0.0		
	NO	1		
40-45	MN			
	SD			
	NO			
45-50	MN			
	SD			
	NO			

Table 22. Mean percent of sample passing 200 (mesh) sieve.

## Land Resource Units

DEPTH		A1	A3	A6	A8	B	C	E	F	F11
0-5	MN	7.1	39.3	53.9	73.5	36.0	5.4	52.3		8.0
	SD	2.1	11.7	9.4	13.6	23.2	3.6	24.1		0.0
	NO	15	13	20	49	13	9	12		1
5-10	MN	11.8	34.2	55.6	76.9	40.1	7.3	39.2		
	SD	9.8	18.5	14.2	16.0	26.8	6.5	21.9		
	NO	17	21	24	50	11	4	5		
10-15	MN	12.6	34.2	54.4	77.1	66.5	7.0	36.0	50.0	40.0
	SD	10.4	19.0	20.4	15.4	31.5	5.4	21.7	3.0	0.0
	NO	14	42	20	38	2	5	8	2	1
15-20	MN	17.0	22.6	54.4	78.6	42.0	11.5			35.0
	SD	0.0	17.6	13.8	15.3	0.0	1.5			33.0
	NO	1	54	16	23	1	2			2
20-25	MN	68.0	24.2	59.4	71.7		3.5			
	SD	0.0	16.3	13.0	22.7		1.5			
	NO	1	31	7	16		2			
25-30	MN		25.6	53.3	71.1	2.0				
	SD		18.6	18.9	23.2	0.0				
	NO		19	6	11	1				
30-35	MN	38.0	26.6	49.0	99.0		1.0			
	SD	9.0	10.7	0.0	0.0		0.0			
	NO	2	7	1	2		2			
35-40	MN		29.2	58.2	83.3					
	SD		22.5	19.2	5.0					
	NO		11	5	3					
40-45	MN		27.0	83.0	71.4		1.0			
	SD		11.5	0.0	14.1		0.0			
	NO		6	1	5		2			
45-50	MN		13.0		80.0		1.0			
	SD		4.9		17.1		0.0			
	NO		5		4		1			

Table 23. Mean void ratio (volume voids/volume solids).  
Land Resource Units

DEPTH		A3	A6	A8	B
0-5	MN				
	SD				
	NO				
5-10	MN		.6		
	SD		0.0		
	NO		1		
10-15	MN		.5	.8	
	SD		0.0	0.0	
	NO		1	1	
15-20	MN	.7	.8	.7	
	SD	0.0	0.0	.1	
	NO	1	1	3	
20-25	MN			.8	.7
	SD			.0	0.0
	NO			2	1
25-30	MN			.6	
	SD			0.0	
	NO			2	
30-35	MN			.7	.8
	SD			.1	0.0
	NO			3	1
35-40	MN		.5	.7	
	SD		0.0	0.0	
	NO		1	1	
40-45	MN				.7
	SD				0.0
	NO				1
45-50	MN			.9	
	SD			0.0	
	NO			1	

Table 24. Mean vertical or horizontal permeability (cm/sec).  
Land Resource Units

DEPTH		A3	A6	A8	B
0-5	MN		.0		
	SD		0.0		
	NO		1		
5-10	MN		.0	.0	
	SD		0.0	.0	
	NO		1	3	
10-15	MN			.0	.0
	SD			.0	.0
	NO			5	3
15-20	MN	.0	.0	.0	.0
	SD	0.0	0.0	.0	.0
	NO	1	1	2	2
20-25	MN		.0		.0
	SD		0.0		.0
	NO		1		4
25-30	MN	.0	.0	.0	
	SD	.0	0.0	0.0	
	NO	2	1	1	
30-35	MN				.0
	SD				.0
	NO				2
35-40	MN	.0			.0
	SD	.0			.0
	NO	2			2
40-45	MN	.0	.0		
	SD	0.0	0.0		
	NO	1	1		
45-50	MN			.0	
	SD			0.0	
	NO			1	



Table 26. Characteristic range<sup>†</sup> of means for engineering properties of land resource units.

	Land Resource Units									
	A1	A3	A6	A8	B	C	E	F	F11	
Unit dry weight (pounds per ft <sup>3</sup> )	94.6 -107.2*	99.0 -108.6	107.7 -110.1	96.1 -100.8	91.2 -103.5*	---	94.8 -102.2	96.0*-108.0*	93.0*-117.0*	
Natural moisture content (percent dry weight)	17.0*- 28.0*	16.6 - 26.6	15.4 - 30.5	23.9 - 28.1	19.7 - 24.9	---	21.5 - 27.0	37.9 - 53.7*	14.0*- 30.0*	
In-place vane shear (tons per ft <sup>2</sup> )	0.3*- 1.2*	1.4*	---	0.4 - 1.2*	1.7*	---	0.3*- 0.6*	0.1*- 0.8*	0.1*- 1.0*	
Standard penetrometer (blows per ft)	7.3 - 46.3	16.5 - 44.1	17.7 - 35.7	9.9 - 32.9	2.0*- 15.0*	20.4 -55.8	8.3 - 11.3	13.1*- 40.0*	11.4*- 16.3*	
TMØ cone penetrometer (blows per ft)	---	37.3 - 85.8	20.4*- 49.7*	9.8*- 33.3	---	7.3*-76.0*	6.0*- 10.0*	4.0*- 9.0*	---	
Unconfined compression Q <sub>u</sub> (tons per ft <sup>2</sup> )	0.1*- 1.8*	1.7 - 2.4	3.1 - 4.5	1.6 - 2.2	1.6 - 2.0	---	0.8 - 1.2	0.1*- 0.5*	0.1*- 1.0*	
Triaxial shear Q <sub>c</sub> (tons per ft <sup>2</sup> )	0.1*- 0.4*	0.2*- 0.9*	0.3*- 1.5*	0.3*- 1.1*	0.4*- 1.2*	---	---	0.4*- 0.6*	0.6*	
Triaxial phi (degrees)	23.0*- 37.0*	11.3*- 34.3*	7.1*- 39.0*	12.0*- 25.7*	7.9*- 13.1*	---	---	22.5*- 35.0*	23.5*	
Hand penetrometer (tons per ft <sup>2</sup> )	---	0.6*- 3.4 *	1.5*- 2.3*	2.1 - 3.6	---	---	2.4*- 2.7*	0.2*- 0.4*	2.8*	
Absorption swell (percent volume)	---	1.2*	1.5*- 3.9*	3.5 - 4.0	3.5*	---	---	---	---	
Absorption pressure (tons per ft <sup>2</sup> )	---	0.3*- 0.5*	0.4*- 2.6*	1.2 - 3.2	1.2*	---	---	---	---	
Linear dry shrinkage (percent distance)	4.9*- 5.1*	2.7*- 9.5*	5.3*- 21.0*	15.9 - 24.2*	18.0*- 21.5*	---	7.1*- 10.0*	---	---	
Liquid limit (percent dry weight)	22.5*- 31.0*	27.6 - 40.7*	38.3*- 59.4*	52.9 - 67.9	29.0*- 54.0*	---	31.0*- 51.2*	42.7 - 63.1	41.0*- 64.0*	
Plastic limit (percent dry weight)	14.0*- 16.7*	15.3*- 20.5	18.3 - 20.3*	21.6 - 26.0	19.8*- 23.0*	---	15.0*- 20.6	22.4 - 28.3	15.0*- 18.0*	
Plastic index	8.0*- 14.2*	9.0 - 31.5	19.9 - 22.8	30.8 - 42.6	20.4*- 24.0*	---	23.0 - 31.0*	20.3 - 34.8	26.0*- 46.0*	
Percent gravel in sample	0.0 *	0*- 6.0	---	0*- 3.0*	---	0.0	1.0*	---	0*	
Percent sand in sample	91.3*- 92.0*	75.7*- 90.0*	---	32.0*- 47.7*	---	98.0 -99.0*	26.0*	---	65.0*	
Percent silt in sample	---	12.0*- 13.0*	13.7*- 29.7*	---	---	---	28.0*	---	---	
Percent clay in sample	---	4.0*- 9.0*	31.3*- 44.2*	---	---	---	45.0*	---	---	
Percent sample passing 200 sieve	7.1 - 12.6	22.6 - 39.3	53.3 - 59.4*	71.1 - 99.0*	36.0 - 40.1	1.0*-11.5*	36.0*- 52.3	50.0*	8.0*- 40.0*	
Vertical or horizontal permeability (cm/sec)	---	---	---	0.0	0.0	---	---	---	---	
Void ratio (volume voids/volume solids)	---	0.7*	0.5*- 0.8*	0.6*- 0.9*	0.7*- 0.8*	---	---	---	---	

<sup>†</sup>Determined by inspection.

\*Indicates means calculated from fewer than 10 test values.

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## APPENDIX A

## DESCRIPTION OF THE ENGINEERING TESTS

For additional information on these tests, refer to Terzaghi and Peck (1967), and Krynine and Judd, (1957).

## UNIT DRY WEIGHT

The unit dry weight is the ratio of the oven-dried sample weight to the volume of the sample expressed in pounds per cubic foot.

$$U. D. W. = \frac{\text{Dry weight of sample in pounds}}{\text{Volume of sample in cubic feet}}$$

## NATURAL MOISTURE CONTENT

The natural moisture content is the ratio of the weight of the water in the sample at the time it was removed from the ground to the oven-dried weight of the sample. The result is expressed as a percentage and is given by the following formula:

$$N. M. C. = \frac{\text{Weight of sample wet} - \text{Weight of sample dry}}{\text{Weight of sample dry}}$$

The natural moisture content will, of course, vary from place to place in similar soils and from time to time even where the same area is sampled repeatedly.

## ATTERBURG LIMITS

Liquid Limit

The liquid limit (LL) is the maximum water content of a soil before it changes from the plastic to the liquid state and is expressed as a percentage of the dry weight of the sample.

$$LL = \frac{\text{Weight of water in sample at liquid state}}{\text{Weight of oven-dried sample}} \times 100$$

### Plastic Limit

The plastic limit (PL) is the minimum water content necessary for a soil to change from the solid to the plastic state. Soil is considered in the plastic state when it will rapidly and permanently deform without cracking. The plastic limit is expressed as a percentage of the dry weight of the sample.

$$PL = \frac{\text{Weight of water in sample at plastic state}}{\text{Weight of oven-dried sample}} \times 100$$

### Plasticity Index

The plasticity index is a method of expressing the relative range of water content through which the soil will exhibit plasticity (being in a plastic state). The larger the value of the plasticity index of the soil, the smaller the range of resistance of that soil to remolding, and thus the greater the probability that the soil will be less desirable for construction. The plasticity index (PI) is stated as the numerical difference between the liquid limit and the plastic limit of a soil.

$$PI = LL - PL$$

### LINEAR DRYING SHRINKAGE

Linear drying shrinkage (LS) is the decrease in one dimension of the soil mass when the water content is reduced from the liquid limit to the shrinkage limit of the soil. The shrinkage limit is the maximum calculated water content at which further loss of water will not cause a decrease in the volume of the soil sample. Linear shrinkage is expressed as the percentage of shrinkage and is calculated as follows:

$$LS = \frac{\text{Length of wet soil bar} - \text{Length of dry soil bar}}{\text{Length of wet soil bar}} \times 100$$

OR

$$LS = \left[ 1 - \sqrt[3]{1 - \frac{\text{Volumetric shrinkage}}{100}} \right] \times 100$$

## TRIAXIAL SHEAR TEST

A triaxial shear test is performed to determine the shear strength of material having a significant sand content or lacking cohesion. The sample is subjected to confining pressure on all sides in addition to an axial load until the sample fails. The shearing strength is found by using Coulomb's equation:

$$s = c + p \tan \phi$$

where

s = Shearing strength expressed as either tons per square foot or pounds per square inch

c = Cohesion constant

p = Normal stress on the surface of sliding

$\phi$  = Angle of internal friction

and Mohr's diagram.

The values reported in this study are c, expressed in tons per square foot, and phi ( $\phi$ ), expressed in degrees.

## UNCONFINED COMPRESSION TEST

An unconfined compression test is used to determine the shear strength of materials with significant clay content. The test is performed by increasing the axial load to failure and is run sufficiently fast to prevent significant drainage or consolidation. The axial stress at failure is the unconfined compressive strength ( $Q_u$ ). The shear strength ( $C_q$ ) is determined as follows:

$$C_q = \frac{1}{2} Q_u$$

Results are reported in tons per square foot.

## POCKET OR HAND PENETROMETER

The pocket penetrometer is a direct-reading, hand-held, calibrated, spring-loaded penetrometer. It is pushed with steady vertical pressure into the soil surface until the piston needle reaches the calibration groove (approximately  $\frac{1}{8}$  inch). The readings are unconfined compression strength values and are accurate to approximately  $\pm 20$  percent of the reading up to a maximum of 4.5 tons per square foot.

## STANDARD PENETROMETER

The standard penetrometer test is the total number of blows a 140-pound hammer falling 30 inches takes to drive a split-spoon sampler to a depth of one foot or to refusal. Refusal is defined as either penetration of less than one foot per 100 blows or penetration of less than six inches per 50 blows. The values are expressed as blows per foot for values less than refusal, and inches per 50 or 100 blows for values greater than refusal.

## THD CONE PENETROMETER

The Texas Highway Department (THD) cone penetrometer test consists of driving a 3-inch diameter cone with a 170-pound hammer dropped from a height of two feet. Values are expressed either as blows per foot (up to 100 blows) or as inches per 100 blows, for the same reasons described above.

## IN-PLACE VANE SHEAR

The in-place vane test consists of pushing a bladed vane into undisturbed material, rotating the vane, and measuring the torsion (in tons/ft<sup>2</sup>) required to fail the cylindrical surface area of soil being sheared by the vanes (Texas Highway Department, 1964).

## PASSING 200 MESH

The passing 200 mesh test is a determination (by grain size) of the amount of clay and silt present in the sample. For particles to pass the no. 200 sieve, two dimensions of the grain must be smaller than 0.074 millimeters. The value is expressed as the percentage of the total sample.

$$\text{Pass 200} = 100 - \frac{\text{Dry weight of sample wash}}{\text{Dry weight before sample washing}}$$

## VOID RATIO

The void ratio is the ratio of the volume of voids (nonsolid material) to the volume of solid material. It is a dimensionless number.

$$c = \frac{\text{Total volume of voids}}{\text{Total volume of the sample} - \text{Total volume of the voids}}$$

APPENDIX B  
COMPUTER PROGRAMS FOR QUANTIFICATION  
OF LAND RESOURCE UNITS

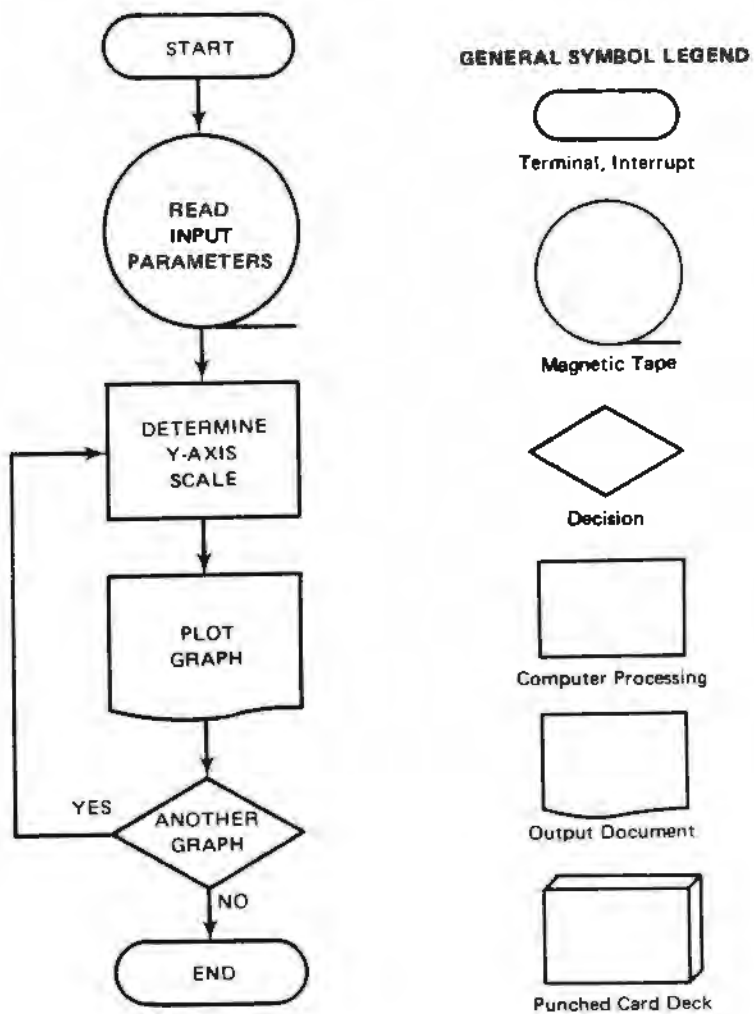


Figure B-1. Generalized flow chart for program EGRAPH.



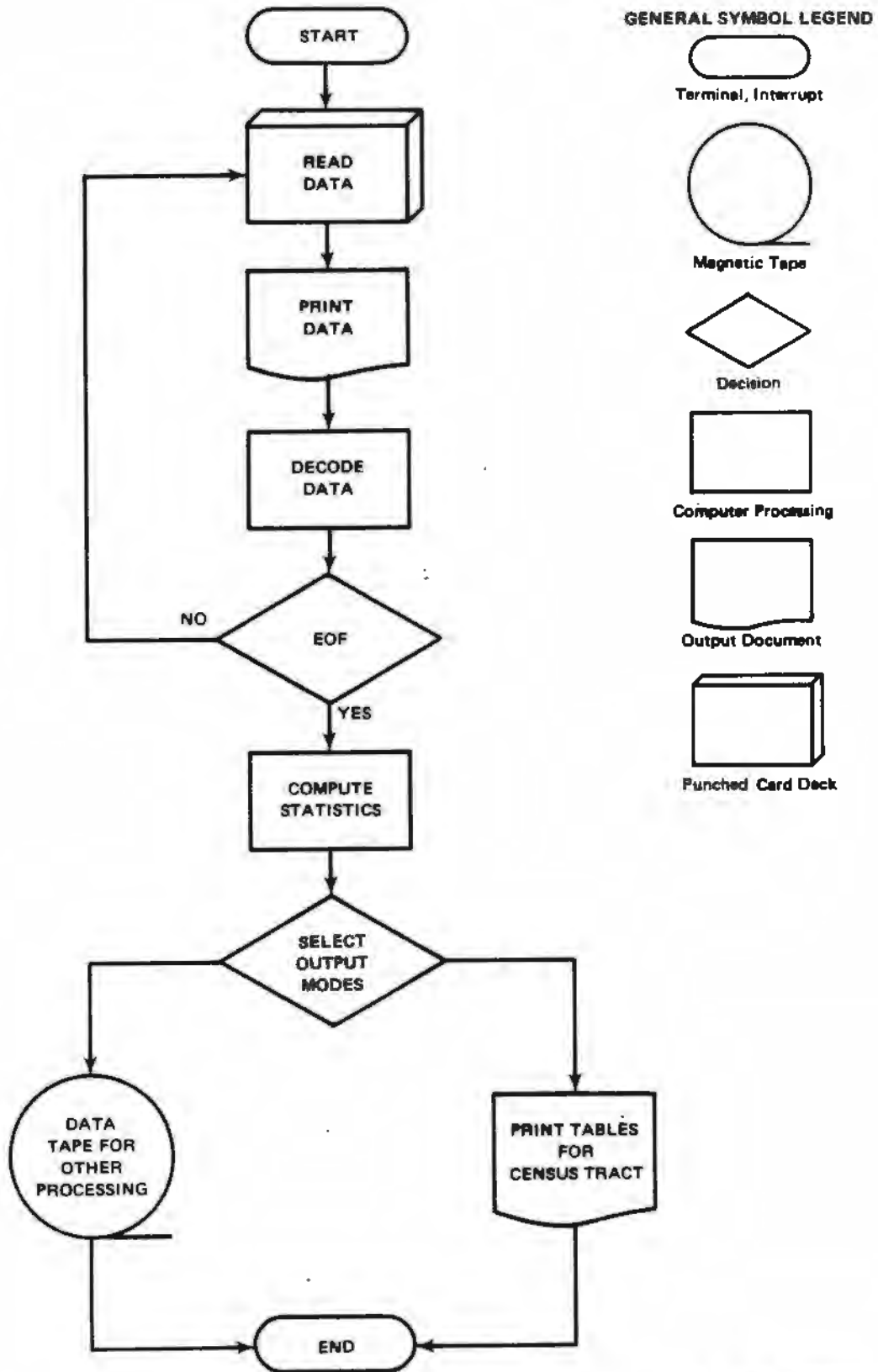


Figure B-2. Generalized flow chart for program ENGTEST.

C	-----	2
	PROGRAM ENGTEST(INPUT,OUTPUT,TAPE50,TAPE60)	4
C	-----	6
C		8
C		10
C	THIS PROGRAM RELATES ENGINEERING TEST PROPERTIES, DEPTH, AND	12
C	RESOURCE CAPABILITY UNITS. THE PROGRAM HAS VARIOUS OUTPUT	14
C	CONTROLLED BY THE DATA HEADER CARD. THE OUTPUT INCLUDES	16
C	A BINARY TAPE FOR THE GRAPHICS PROGRAM, A QUICK REFERENCE	18
C	TABLE FOR ALL VALUES 0-5 FEET, A COMPLETE LISTING OF THE	20
C	DATA, STATISTICAL TABLES FOR EACH ENGINEERING TEST, INVENTORY	22
C	SUMMARY OF THE RESOURCE CAPABILITY UNITS AND THE ENGINEERING	24
C	FOUND IN THE DATA, BCD TAPE FOR INPUT TO BMD CORRELATION	26
C	MATRIX PROGRAM.	28
C		30
C	S1 -- YIELDS MEANS FOR GRAPHING	32
C	S2 -- YIELDS QUICK REFERENCE TABLE	34
C	S3 -- YIELDS DATA	36
C	S4 -- YIELDS TABLES	38
C	S5 -- YIELDS VARIABLE TOTALS	40
C	S6 AND S7 -- YIELDS VALUES FROM UNIT S7 FOR CORRELATION MATRIX	42
C		44
C	THIS PROGRAM IS DESIGNED TO BE COMPILED BY CDC FORTRAN RUN	46
C	60.2 COMPILER AND TO RUN ON THE UNIVERSITY OF TEXAS AT AUSTIN	48
C	CDC 6400/6600 COMPUTER OPERATING UNDER THE UT-20 SYSTEM.	50
C	SOME CODE MAY BE UNIQUE TO THE SYSTEM.	52
C		54
C	TEXAS BUREAU OF ECONOMIC GEOLOGY--DLR--5/31/74.	56
C		58
C	-----	60

	C		62
	C	RESOURCE CAPABILITY UNITS FOUND IN DATA	64
	C		66
000002		REAL R(9)	68
000002		DATA R/2HA1,2HA3,2HA6,2HAR,1HR,1HC,1HE,1HF,3HF11/	70
	C		72
	C	VARIABLE FORMATTING DATA	74
000002		REAL BLANKS,FSPEC,ASPEC,FSPEC2	76
000002		REAL MNROW,SDROW1,SDROW2,NOROW,FORM(20)	78
000002		INTEGER K2,K3,K4	80
000002		DATA MNROW/10H(*0*8X*MN*/	82
000002		DATA SDROW1,SDROW2/10H(* *12*-* ,9H12,2X*SD*/	84
000002		DATA NOROW/9H* *8X*NO*/	86
000002		DATA BLANKS,FSPEC,ASPEC,FORM(20)/7H                    ,5HF7,1,,3HA7,,1H)/	88
000002		DATA FSPEC2/9H2X,F4,1X,/	90
000002		DATA RRR/2H /	92
000002		REAL HEAD1(120),PVALUE(17),RR(9)	94
000002		REAL EP(23)	96
000002		REAL SUM(9,12,23),SUM2(9,12,23),MEAN(9,12,23),SD(9,12,23)	98
000002		REAL FOUND1,FOUND2,SSWELL1,SSWELL2,PERM	100
000002		INTEGER U,0,DD,DD0,T,DCODE,L,K,KOUNT(9,12,23),K1	102
000002		INTEGER IFLAG,NSGRID,EWGRID,SURELEV,DEPTH,UDRYWT,NMOIST	104
000002		INTEGER FTEST1,FTEST2,SSTEST1,SSTEST2,LIQUID,PLASTIC,PINDEX	106
000002		INTEGER GRAVEL,SAND,SILT,CLAY,PCOEF,MASGRID,NOTEST	108
000002		INTEGER S1,S2,S3,S4,S5,S6	110
000002		REAL S7	112
	C		114
	C	HEADING FOR TABULAR DATA	116

000002	C	DATA (HEAD1(L),L=76)/10HUNIT DRY W,10HEIGHT (POU,10HNDS PER CU,	118
	1	10HBIC FOOT) ,10HNATURAL MO,10HISTURE CON,10HTENT (PER,	120
	2	10HCENT D WT),10HIN PLACE V,10HANE SHEAR ,10H (TONS PER,	122
	3	10H SQ. FT.) ,10HSTANDARD P,10HENETROMETE,10HR (BLOWS ,	124
	4	10HPER FOOT) ,10HTHO CONE P,10HENETROMETE,10HR (BLOWS ,	126
	5	10HPER FOOT) ,10HUNCONFINED,10H COMPRESSI,10HON Q(U) (T,	128
	6	10HONS/SQ,FT),10HTRIAxIAL S,10HHEAR Q(C) ,10H (TONS PER,	130
	7	10H SQ. FT.) ,10HTRIAxIAL P,10HMI (DEGRE,10HES) ,	132
	8	10H ,10HHAND PENET,10HROMETER (T,10HONS PER SQ,	134
	9	10HUARE FOOT) ,10HSHRINKAGE ,10HLIMIT (PE,10HRCENT DRY ,	136
	A	10HWEIGHT) ,10HABSORPTION,10H SWELL (P,10HERCENT VOL,	138
	B	10HUME) ,10HABSORPTION,10H PRESSURE ,10H (TONS PER,	140
	C	10H SQ. FT.) ,10HLINEAR DRY,10H SHRINKAGE,10H (PERCENT,	142
	D	10H DISTANCE),10HLIQUID LIM,10HIT (PERCE,10HNT DRY WEI,	144
	E	10HGHT) ,10HPLASTIC LI,10HMIT (PERC,10HENT DRY WE,	146
	F	10HIGHT) ,10HPLASTIC IN,10HDEX (PERC,10HENT DRY WE,	148
	G	10HIGHT) ,10HPERCENT GR,10HAVEL IN SA,10HMPLE .	150
	H	10H ,10HPERCENT SA,10HND IN SAMP,10HLE .	152
	I	10H ,10HPERCENT SI,10HLT IN SAMP,10HLE .	154
	J	10H /	156
000002		DATA (HEAD1(L),L=77,92)/10HPERCENT CL,10HAY IN SAMP,10HLE ,	158
	1	10H ,10HPERCENT SA,10HMPLE PASSI,10HNG 200 SIE,	160
	2	10HVE ,10HVERT. OR H,10HORZ. PERME,10HABILITY (,	162
	3	10HCM/SEC) ,10HVOID RATIO,10H (VOLUME ,10HVOIDS/VOLU,	164
	4	10HME SOLIDS)/	166
			168
	C	READ DATA HEADER CARD	170
	C		172
	C		174
000002		READ 510, S1,S2,S3,S4,S5,S6,S7	176
	C		178

	C	READ NUMBER OF TEST TO BE CONSIDERED	180
	C		182
000024		READ 520, NOTEST	184
000032		K=0	186
000033		ITOT=0	188
000033		ITOT2=0	190
000034		NUM=0	192
000034		K1=0	194
000035		NUMUNIT=0	196
	C		198
	C	VARIABLE ZERO SET	200
	C		202
000036		DO 110 U=1,9	204
000037		DO 110 D=1,12	206
000040		DO 110 T=1,NOTEST	208
000051		SUM(U,D,T)=0.0	210
000052		SUM2(U,D,T)=0.0	212
000052		KOUNT(U,D,T)=0	214
000053		MEAN(U,D,T)=0.0	216
000053		SD(U,D,T)=0.0	218
	C		220
000053	110	CONTINUE	222
	C		224
000061	120	CONTINUE	226
	C		228
000061		DO 130 I=1,NOTEST	230
000066		EP(I)=0.0	232

000067	C	130	CONTINUE	234
	C			236
	C		READ DATA CARD	238
	C			240
000070			READ 530, RUNIT, MASGRID, NSGRID, EWGRID, CORENO, SOURCE, BIBLIO,	242
		1	SURELEV, DEPTH, UDRYWT, NMOIST, FTEST1, FOUND1, FTEST2, FOUND2,	244
		2	SSTEST1, SSWELL1, SSTEST2, SSWELL2, LIQUID, PLASTIC, PINDEX,	246
		3	GRAVEL, SAND, SILT, CLAY, PERM, PCOEF, EP(23)	248
000165			IF (S3.EQ.0) GO TO 160	250
000166			K1=K1+1	252
000170			IF (K1.EQ.1) PRINT 540	254
000174			IF (K1.EQ.55) K1=0	256
000177			IF (EWGRID.EQ.0.AND.NSGRID.EQ.0) GO TO 180	258
	C			260
	C		PRINT DATA	262
	C			264
000205			PRINT 550, RUNIT, MASGRID, NSGRID, EWGRID, CORENO, SOURCE, BIBLIO,	266
		1	SURELEV, DEPTH	268
000232			IF (UDRYWT.NE.0) PRINT 560, UDRYWT	270
000241			IF (NMOIST.NE.0) PRINT 570, NMOIST	272
000250			IF (FTEST1.NE.0) PRINT 580, FTEST1, FOUND1	274
000261			IF (FTEST2.NE.0) PRINT 590, FTEST2, FOUND2	276
000272			IF (SSTEST1.NE.0) PRINT 600, SSTEST1, SSWELL1	278
000303			IF (SSTEST2.NE.0) PRINT 610, SSTEST2, SSWELL2	280
000314			IF (LIQUID.NE.0.OR.PLASTIC.NE.0) PRINT 620, LIQUID, PLASTIC, PINDEX	282
000333			IF (.NOT.CLAY.AND.CLAY.LT.7) GO TO 140	284
000340			IF (GRAVEL.NE.0) PRINT 630, GRAVEL	286
000347			IF (SAND.NE.0) PRINT 640, SAND	288
000356			IF (SILT.NE.0) PRINT 650, SILT	290
000365			IF (CLAY.NE.0) PRINT 660, CLAY	292
				294

000374	C	GO TO 150	296
			298
000375	C	140 CONTINUE	300
			302
000375	C	PRINT 670, GRAVEL, SAND, SILT, CLAY	304
			306
000411	C	150 CONTINUE	308
			310
000411	C	IF (PERM.NE.0) PRINT 680, PERM, PCOEF	312
000422		IF (EP(23).NE.0.) PRINT 690, EP(23)	314
000431		IF (LIQUID-PLASTIC.NE.PINDEX) PRINT 700	316
			318
000437	C	160 CONTINUE	320
			322
	C	DECODE DATA	324
	C		326
	C		328
000437		DO 170 I=1,2	330
000445		IF (FTEST1.GT.0.AND.FOUND1.EQ.0.0) FOUND1=.0000000001	332
000455		IF (FTEST1.EQ.1) EP(3)=FOUND1	334
000460		IF (FTEST1.EQ.3) EP(4)=FOUND1	336
000463		IF (FTEST1.EQ.4) EP(5)=FOUND1	338
000466		IF (FTEST1.EQ.6) EP(6)=FOUND1	340
000471		IF (FTEST1.EQ.7) EP(8)=FOUND1	342
000474		IF (FTEST1.EQ.8) EP(9)=FOUND1	344
000477		FTEST1=FTEST2	346
000500		FOUND1=FOUND2	348

000501		IF (EP(6).GT.0.0.AND. EP(8).GT.0.0) EP(7)=EP(6)	350
000511		IF (EP(7).GT.0.0) EP(6)=0.0	352
000514		IF (SSTEST1.EQ.1) EP(10)=SSWELL1	354
000517		IF (SSTEST1.EQ.3) EP(11)=SSWELL1	356
000522		IF (SSTEST1.EQ.4) EP(12)=SSWELL1	358
000525		IF (SSTEST1.EQ.5) EP(13)=SSWELL1	360
000530		SSTEST1=SSTEST2	362
000531		SSWELL1=SSWELL2	364
	C		366
000532	170	CONTINUE	368
	C		370
000533		IF	372
	1	(.NOT.GRAVEL.AND..NOT.SAND.AND..NOT.CLAY.AND.GRAVEL.LT.1	374
	2	.AND.SAND.LT.1.AND.CLAY.LT.1) EP(21)=SILT	376
000552		IF (SAND.NE.0.AND..NOT.CLAY.AND.CLAY.LT.1) EP(21)=SILT	378
000563		IF (EP(21).GT.0.0) SILT=0	380
000566		EP(1)=UDRYWT	382
000567		EP(2)=NMOIST	384
000571		EP(14)=LIQUID	386
000572		EP(15)=PLASTIC	388
000574		EP(16)=PINDEX	390
000575		IF (LIQUID=PLASTIC.EQ.0.AND.LIQUID.NE.0) EP(16)=.0000000001	392
000604		EP(17)=GRAVEL	394
000605		IF (GRAVEL.EQ.0.AND.SAND.NE.0) EP(17)=.0000000001	396
000614		EP(18)=SAND	398
000615		EP(19)=SILT	400
000617		EP(20)=CLAY	402
000620		IF (EP(19)+EP(20).GT.0.0.AND. EP(21).EQ.0.0) EP(21)=EP(19)+EP(20)	404
000631		PMULT=10,**(-1*PCOEF)	406
000637		EP(22)=PERM*PMULT	408



000641	C	180	CONTINUE	410
	C			412
	C		CHECK FOR EOD	414
	C			416
000641			IF (EWGRID.EQ.0.AND.NSGRID.EQ.0) GO TO 250	418
000647			DO 190 U=1,9	420
000654			IF (RUNIT.EQ.R(U)) GO TO 200	422
000656		190	CONTINUE	424
000660			PRINT 710, RUNIT	426
000665			GO TO 240	428
000666		200	CONTINUE	430
	C			432
	C		CLASSIFY DEPTHS INTO 5 FOOT CLASSES	434
	C			436
000666			DO 210 DCODE=5,60,5	438
000673			IF (DEPTH.GE.(DCODE-5).AND.DEPTH.LT.DCODE) GO TO 220	440
000702		210	CONTINUE	442
000703			PRINT 720	444
000707			GO TO 240	446
000710		220	CONTINUE	448
000710			D=DCODE/5	450
000713			IF (S6.EQ.0) GO TO 230	452
000715			IF (RUNIT.NE.S7) GO TO 230	454
000717			DEEP=FLOAT(DEPTH)	456
	C			458
	C		WRITE VALUES FOR CORRELATION MATRIX	460
	C			462
				464

000720		WRITE (60,730) DEEP,(EP(K9),K9=1,NOTEST)	466
000731		NUMUNIT=NUMUNIT+1	468
000733	230	CONTINUE	470
000733		IF (S1.EQ.0.AND.S2.EQ.0.AND.S4.EQ.0) GO TO 240	472
	C		474
	C	CALCULATE MEAN, NUMBER OF SAMPLES AND STANDARD DEVIATION	476
	C		478
000743		DO 240 T=1,NOTEST	480
000744		SUM(U,D,T)=SUM(U,D,T)+EP(T)	482
000752		SUM2(U,D,T)=SUM2(U,D,T)+EP(T)**2	484
000756		IF (EP(T).EQ.0,0) GO TO 240	486
000763		NUM=NUM+1	488
000765		KOUNT(U,D,T)=KOUNT(U,D,T)+1	490
000766		MEAN(U,D,T)=SUM(U,D,T)/KOUNT(U,D,T)	492
000770		IF (KOUNT(U,D,T).EQ.1) GO TO 240	494
000775		ZCHECK=(FLOAT(KOUNT(U,D,T))*SUM2(U,D,T))-(SUM(U,D,T)**SUM(U,D,T))	496
001010		IF (ZCHECK.LT.0) GO TO 240	498
001011		SD(U,D,T)=(SQRT(ZCHECK))/FLOAT(KOUNT(U,D,T))	500
001025	240	CONTINUE	502
001030		IF (EWGRID.NE.0.AND.NSGRID.NE.0) GO TO 120	504
001036	250	CONTINUE	506
001036		IF (S5.EQ.0) GO TO 280	508
	C		510
	C	PRINT DATA INVENTORY SUMMARIES	512
	C		514
001037		PRINT 740	516
001043		PRINT 750, NUM	518
001051		DO 280 T=1,NOTEST	520
001053		PRINT 760, HEAD1(4*T-3),HEAD1(4*T-2),HEAD1(4*T-1),HEAD1(4*T)	522
001072		DO 270 U=1,9	524
001074		ITOT=0	526
001075		DO 260 D=1,12	528
001109		ITOT=ITOT+KOUNT(U,D,T)	530

001106	260	CONTINUE	532
001107		PRINT 770, R(U),ITOT	534
001117		ITOT2=ITOT2+ITOT	536
001121	270	CONTINUE	538
001123		PRINT 780, ITOT2	540
001130		ITOT2=0	542
001131	280	CONTINUE	544
001134		IF (S1.EQ.0) GO TO 300	546
	C		548
	C	WRITE MEANS FOR GRAPHICS PROGRAM	550
	C		552
001135		CALL IOP (2HNB,50,R,9)	554
001140		CALL IOP (2HNB,50,NOTEST,1)	556
001143		DO 290 U=1,9	558
001145		DO 290 D=1,12	560
001146		DO 290 T=1,NOTEST	562
001147		CALL IOP (2HNB,50,MEAN(U,D,T),1)	564
001156	290	CONTINUE	566
001165		CALL IOP (2HWF,50)	568
001167	300	CONTINUE	570
001167		IF (S4.EQ.0) GO TO 470	572
	C		574
	C	PRINT STATISTICAL TABLES	576
	C		578
001170		DO 460 T=1,NOTEST	580

001172	DO 310 KK=1,9	582
001177	RR(KK)=RRR	584
001177	310 CONTINUE	586
001200	K2=1	588
001201	DO 330 U=1,9	590
001203	DO 330 D=1,12	592
001204	IF (KOUNT(U,D,T).EQ.0) GO TO 330	594
001211	IF (K2.EQ.1) GO TO 320	596
001213	IF (RR(K2).EQ.R(U).OR.RR(K2-1).EQ.R(U)) GO TO 330	598
001223	320 CONTINUE	600
001223	RR(K2)=R(U)	602
001226	K2=K2+1	604
001227	330 CONTINUE	606
001233	K2=K2-1	608
001235	PRINT 790, T, HEAD1(4*T-3), HEAD1(4*T-2), HEAD1(4*T-1), HEAD1(4*T)	610
001256	PRINT 800, (RR(J), J=1, K2)	612
001265	DO 460 D=1,12	614
001267	K3=1	616
001270	DO 340 U=1,9	618
001271	IF (RR(K3).NE.R(U)) GO TO 340	620
001300	FORM(K3+1)=FSPEC	622
001301	PVALUE(K3)=MEAN(U,D,T)	624
001303	IF (MEAN(U,D,T).EQ.0.0) FORM(K3+1)=ASPEC	626
001311	IF (MEAN(U,D,T).EQ.0.0) PVALUE(K3)=BLANKS	628
001317	K3=K3+1	630
001321	340 CONTINUE	632
001323	IF (K3.GT.20) STOP 1	634
001330	IF (K3.LT.17) GO TO 350	636
001333	GO TO 370	638
001333	350 CONTINUE	640

001333		DO 360 K4=K3,17	642
001341		PVALUE(K4)=BLANKS	644
001342		FORM(K4+1)=ASPEC	646
001342	360	CONTINUE	648
001343	370	CONTINUE	650
001343		FORM(1)=MNR0W	652
001345		FORM(19)=BLANKS	654
001346		PRINT FORM, PVALUE	656
001356		K3=1	658
001357		DD=(D*5)-5	660
001360		DDD=D*5	662
001362		DO 380 U=1,9	664
001363		IF (RR(K3).NE.R(U)) GO TO 380	666
001372		FORM(K3+2)=FSPEC	668
001373		PVALUE(K3)=SD(U,D,T)	670
001375		IF (SD(U,D,T).EQ.0.0.AND.KOUNT(U,D,T).EQ.0) FORM(K3+2)=ASPEC	672
001413		IF (SD(U,D,T).EQ.0.0.AND.KOUNT(U,D,T).EQ.0) PVALUE(K3)=BLANKS	674
001432		K3=K3+1	676
001434	380	CONTINUE	678
001436		IF (K3.GT.20) STOP 2	680
001443		IF (K3.LT.17) GO TO 390	682
001446		GO TO 410	684
001446	390	CONTINUE	686
001446		DO 400 K4=K3,17	688
001454		PVALUE(K4)=BLANKS	690
001455		FORM(K4+2)=ASPEC	692
001455	400	CONTINUE	694
001456	410	CONTINUE	696

001456		FORM(1)=SDROW1	698
001460		FORM(2)=SDROW2	700
001461		PRINT FORM, DD,DDD,PVALUE	702
001473		K3=1	704
001474		DO 420 U=1,9	706
001476		IF (RR(K3).NE.R(U)) GO TO 420	708
001505		FORM(K3+1)=FSPEC2	710
001506		PVALUE(K3)=KOUNT(U,D,T)	712
001510		IF (KOUNT(U,D,T).EQ.0) FORM(K3+1)=ASPFc	714
001516		IF (KOUNT(U,D,T).EQ.0) PVALUE(K3)=BLANKS	716
001524		K3=K3+1	718
001526	420	CONTINUE	720
001530		IF (K3.GT.20) STOP 3	722
001535		IF (K3.LT.17) GO TO 430	724
001540		GO TO 450	726
001540	430	CONTINUE	728
001540		DO 440 K4=K3,17	730
001546		PVALUE(K4)=BLANKS	732
001547		FORM(K4+1)=ASPEC	734
001547	440	CONTINUE	736
001550	450	CONTINUE	738
001550		FORM(1)=NOROW	740
001552		FORM(19)=BLANKS	742
001553		PRINT FORM, PVALUF	744
001561	460	CONTINUE	746
001566	470	CONTINUE	748
001566		IF (S2.EQ.0) GO TO 490	750
C			752
C		PRINT QUICK REFERENCE TABLE	754
C			756

```

001567      PRINT 810      758
001573      PRINT 820, R  760
001601      DO 480 T=1,NOTEST 762
001603      PRINT 830, HEAD1(4*T-3),HEAD1(4*T-2),HEAD1(4*T-1),HEAD1(4*T), 764
              1 (MEAN(U,1,T),U=1,9) 766
001626      480 CONTINUE 768
001631      490 CONTINUE 770
001631          IF (S6.EQ.0) GO TO 500 772
001632      PRINT 840, NUMUNIT,S7 774
001642      500 CONTINUE 776
001642          CALL IOP (2HWF,50) 778
C 780
001644      510 FORMAT (6I1,A3) 782
001644      520 FORMAT (I2) 784
001644      530 FORMAT (A3,I1,I6,I5,A5,A1,A2,I4,2I3,I2,4(I1,F4.1),3I3,4I2,F4.2, 786
              1 I1,F3.1) 788
001644      540 FORMAT (1RES M N-S*5X*E-W CORE S B ELEV DPT UDW NM 790
              1 -FOUNDATION-- -SHRINK-SWELL- ATERBERG -GRAIN-SIZE- PERM V 792
              2OID*/* UNIT G CORD CORD NO R RF*22X*C TEST C TEST C T 794
              3EST C TEST LL PL PI GR SD ST CL*10X*RAT*) 796
001644      550 FORMAT (* *A3,2X,I1,2X,I6,2X,I5,2X,A5,2X,A1,2X,A2,2X,I4,2X,I3) 798
001644      560 FORMAT (* **48X,I3) 800
001644      570 FORMAT (* **53X,I2) 802
001644      580 FORMAT (* **57X,I1,1X,F4.1) 804
001644      590 FORMAT (* **64X,I1,1X,F4.1) 806
001644      600 FORMAT (* **73X,I1,1X,F4.1) 808
001644      610 FORMAT (* **80X,I1,1X,F4.1) 810
001644      620 FORMAT (* **88X,3I3) 812

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001644	630	FORMAT	(*00100X,I2)	814
001644	640	FORMAT	(*00103X,I2)	816
001644	650	FORMAT	(*00106X,I2)	818
001644	660	FORMAT	(*00109X,I2)	820
001644	670	FORMAT	(*00100X,4(I2,1X))	822
001644	680	FORMAT	(*00113X,F4.2**I1)	824
001644	690	FORMAT	(*00122X,F3.1)	826
001644	700	FORMAT	(*00127X*PI ERROR*)	828
001644	710	FORMAT	(*00128X*RCU *A3)	830
001644	720	FORMAT	(*00128X*DEPTH ER*)	832
001644	730	FORMAT	(3(8G10.3/))	834
001644	740	FORMAT	(*1*20X*PROGRAM STATISTICS*//)	836
001644	750	FORMAT	(*0 TOTAL ENGINEERING TEST VALUES CONSIDERED*I9)	838
001644	760	FORMAT	(*1 STATISTICS FOR *4A10//9X*RCU)*6X*TOTALS*/)	840
001644	770	FORMAT	(10X,A3,I10)	842
001644	780	FORMAT	(*0 TOTAL VALUES FOR THIS TEST*I7)	844
001644	790	FORMAT	(*1*60X*TABLE *I2 /* *40X*MEAN *4A10)	846
001644	800	FORMAT	(*0*48X*RFSOURCE CAPABILITY UNITS*/* DEPTH* 6X,17(2X,A3,2	848
			1X))	850
001644	810	FORMAT	(*1*20X*QUICK REFERENCE TABLE FOR ALL MEANS BETWEEN 0-5 FE	852
			1ET*)	854
001644	820	FORMAT	(*0*44X,9(3X,A3,2X))	856
001644	830	FORMAT	(*0 *4A10,3X,9F8.1)	858
001644	840	FORMAT	(*1*I8* VALUES FOR UNIT *A3* WRITTEN ON TAPE 60)	860
001644		END		862-



PROGRAM LENGTH INCLUDING I/O BUFFERS  
043142

FUNCTION ASSIGNMENTS

STATEMENT ASSIGNMENTS

120	-	000062	140	-	000376	150	-	000412	160	-	000440
180	-	000642	190	-	000657	200	-	000667	220	-	000711
230	-	000734	240	-	001026	250	-	001037	280	-	001132
300	-	001170	320	-	001224	330	-	001230	340	-	001322
350	-	001334	370	-	001344	380	-	001435	390	-	001447
410	-	001457	420	-	001527	430	-	001541	450	-	001551
470	-	001567	490	-	001632	500	-	001643	510	-	001705
520	-	001707	530	-	001711	540	-	001721	550	-	001752
560	-	001761	570	-	001764	580	-	001767	590	-	001773
600	-	001777	610	-	002003	620	-	002007	630	-	002012
640	-	002015	650	-	002020	660	-	002023	670	-	002026
680	-	002031	690	-	002035	700	-	002040	710	-	002044
720	-	002047	730	-	002053	740	-	002056	750	-	002063
760	-	002072	770	-	002100	780	-	002103	790	-	002110
800	-	002115	810	-	002125	820	-	002135	830	-	002141
840	-	002145									

BLOCK NAMES AND LENGTHS

VARIABLE ASSIGNMENTS

ASPEC	-	002226	BIBLIO	-	033023	BLANKS	-	002224	CLAY	-	033000
CORENO	-	033021	D	-	026063	NCODE	-	026067	DD	-	026064
DDD	-	026065	DEEP	-	033025	DEPTH	-	032763	EP	-	002506
EWGRID	-	032761	FORM	-	002234	FOUND1	-	026055	FOUND2	-	026056

FSPEC	=	002225	FSPEC2	=	002227	FTEST1	=	032766	FTEST2	=	032767
GRAVEL	=	032775	HEAD1	=	002264	I	=	033017	IFLAG	=	032757
ITOT	=	033013	ITOT2	=	033014	J	=	033031	K	=	026071
KK	=	033030	KOUNT	=	026072	K1	=	032756	K2	=	002260
K3	=	002261	K4	=	002262	K9	=	033026	L	=	026070
LIQUID	=	032772	MASGRID	=	033002	MEAN	=	014305	MNROW	=	002230
NMOIST	=	032765	NOROW	=	002233	NOTEST	=	033003	NSGRID	=	032760
NUM	=	033015	NUMUNIT	=	033016	PCOEF	=	033001	PERM	=	026061
PINDEX	=	032774	PLASTIC	=	032773	PMULT	=	033024	PVALUE	=	002454
R	=	002213	RR	=	002475	RRR	=	002263	RUNIT	=	033020
SAND	=	032776	SD	=	021171	SDROW1	=	002231	SDROW2	=	002232
SILT	=	032777	SOURCE	=	033022	SSTEST1	=	032770	SSTEST2	=	032771
SSWELL1	=	026057	SSWELL2	=	026060	SUM	=	002535	SUM2	=	007421
SURELEV	=	032762	S1	=	033004	S2	=	033005	S3	=	033006
S4	=	033007	S5	=	033010	S6	=	033011	S7	=	033012
T	=	026066	U	=	026062	UORYWT	=	032764	ZCHECK	=	033027

START OF CONSTANTS  
001647

START OF TEMPORARIES  
002153

START OF INDIRECTS  
002177

UNUSED COMPILER SPACE  
000400

```

C ----- 2
PROGRAM EGRAPH(INPUT,OUTPUT,TAPE50=INPUT) 4
C ----- 6
C ----- 8
C 10
C THIS PROGRAM PLOTS THE MEANS OF ENGINEERING TESTS FOR EACH 12
C RESOURCE CAPABILITY UNIT AS CALCULATED BY THE PROGRAM ENGTEST. 14
C EACH MEAN IS PLOTTED AS A FUNCTION OF DEPTH WITH ALL VALUES 16
C FOR ONE TYPE ENGINEERING TEST PLOTTED ON A SINGLE GRAPH. 18
C THE PLOTS ARE PRODUCED ON 12 - INCH WIDE PAPER ON THE CALCOMP 20
C PLOTTER. 22
C 24
C THIS PROGRAM IS DESIGNED TO BE COMPILED BY CDC FORTRAN RUN 26
C 60.2 COMPILER AND TO RUN ON THE UNIVERSITY OF TEXAS AT AUSTIN 28
C CDC 6400/6600 COMPUTER OPERATING UNDER THE UT-2D SYSTEM. 30
C SOME CODE MAY BE UNIQUE TO THE SYSTEM. 32
C 34
C 36
C TEXAS BUREAU OF ECONOMIC GEOLOGY--DLB--5/31/74. 38
C 40
C ----- 42
C 44
000002 REAL Y(2*2),XA,YA(3),MEAN(9,12,28) 46
000002 INTEGER U,D,T,R(9) 48
C 50
C READ RCU SYMBOLS 52
C 54
000002 CALL IOP (2HRB,50,R,9) 56
C

```

	C	READ NUMBER OF ENGINEERING TESTS	58
	C		60
000005		CALL IOP (2HRB,50,NOTEST,1)	62
000010		DO 110 U=1,9	64
000012		DO 110 D=1,12	66
000013		DO 110 T=1,NOTEST	68
	C		70
	C	READ MEANS	72
	C		74
	C		76
000014		CALL IOP (2HRB,50,MEAN(U,D,T),1)	78
000023	110	CONTINUE	80
	C		82
	C	INITIALIZE PLOTTER	84
	C		86
000032		CALL BGNPLT	88
	C		90
	C	DETERMINE Y-SCALE FOR GRAPH	92
	C		94
000033		DO 160 T=1,NOTEST	96
000035		I=1	98
000036		DO 120 U=1,9	100
000037		DO 120 D=1,10	102
000050		IF (MEAN(U,D,T).EQ.0.) GO TO 120	104
000051		Y(I)=MEAN(U,D,T)	106
000051		I=I+1	108
000052	120	CONTINUE	110
000057		I=I-1	112
000060		DO 130 K=1,I	114
000066		IF (Y(K).NE.0.) GO TO 140	116

000067	130	CONTINUE	118
000071		GO TO 150	120
000071	140	CONTINUE	122
000071		CALL PLT (1.,0.55,-3)	124
000074		CALL SCALE (Y,10.0,I,1)	126
	C		128
	C	LABEL Y-AXIS	130
	C		132
000077		IF (T.EQ.1) CALL AXIS (-0.25,0.,33HMEAN UNIT DRY WEIGHT (LBS/CU.FT	134
		1.),33,10.,90.0,Y(I+1),Y(I+2),.14.,105,0)	136
000120		IF (T.EQ.2) CALL AXIS (-0.25,0.,44HMEAN NATURAL MOISTURE CONTENT (	138
		1PER. DRY WT.),44,10.,90.0,Y(I+1),Y(I+2),.14.,105,0)	140
000141		IF (T.EQ.3) CALL AXIS (-0.25,0.,38HMEAN IN PLACE VANE SHEAR (TONS/	142
		1SQ.FT.),38,10.,90.0,Y(I+1),Y(I+2),.14.,105,2)	144
000162		IF (T.EQ.4) CALL AXIS (-0.25,0.,38HMEAN STANDARD PENETROMETER (BLO	146
		1WS/FT.),38,10.,90.0,Y(I+1),Y(I+2),.14.,105,0)	148
000203		IF (T.EQ.5) CALL AXIS (-0.25,0.,38HMEAN THD CONE PENETROMETER (BLO	150
		1WS/FT.),38,10.,90.0,Y(I+1),Y(I+2),.14.,105,0)	152
000224		IF (T.EQ.6) CALL AXIS (-0.25,0.,33HMEAN UNCONFINED COMPRESSION -Q(	154
		1U),33,10.,90.0,Y(I+1),Y(I+2),.14.,105,1)	156
000245		IF (T.EQ.7) CALL AXIS (-0.25,0.,19HMEAN TRIAXIAL -Q(C),19,10.,90.0	158
		1,Y(I+1),Y(I+2),.14.,105,1)	160
000266		IF (T.EQ.8) CALL AXIS (-0.25,0.,27HMEAN TRIAXIAL PHI (DEGREES),27,	162
		110.,90.0,Y(I+1),Y(I+2),.14.,105,1)	164
000307		IF (T.EQ.9) CALL AXIS (-0.25,0.,36HMEAN HAND PENETROMETER (TONS/SQ	166
		1.FT.),36,10.,90.0,Y(I+1),Y(I+2),.14.,105,1)	168
000330		IF (T.EQ.10) CALL AXIS (-0.25,0.,35HMEAN SHRINKAGE LIMIT (PER. DRY	170
		1 WT.),35,10.,90.0,Y(I+1),Y(I+2),.14.,105,1)	172
000351		IF (T.EQ.11) CALL AXIS (-0.25,0.,38HMEAN ABSORPTION SWELL (PERCENT	174
		1 VOLUME),38,10.,90.0,Y(I+1),Y(I+2),.14.,105,1)	176
000372		IF (T.EQ.12) CALL AXIS (-0.25,0.,38HMEAN ABSORPTION PRESSURE (TONS	178

	1/SQ.FT.),38,10.,90.0,Y(I+1),Y(I+2),.14,.105,1)	180
000413	IF (T.EQ.13) CALL AXIS (-0.25,0.,44HMEAN LINEAR DRY SHRINKAGE (PER	182
	CENT DISTANCE),44,10.,90.0,Y(I+1),Y(I+2),.14,.105,1)	184
000434	IF (T.EQ.14) CALL AXIS (-0.25,0.,32HMEAN LIQUID LIMIT (PER. DRY WT	186
	1.),32,10.,90.0,Y(I+1),Y(I+2),.14,.105,0)	188
000455	IF (T.EQ.15) CALL AXIS (-0.25,0.,33HMEAN PLASTIC LIMIT (PER. DRY W	190
	1T.),33,10.,90.0,Y(I+1),Y(I+2),.14,.105,0)	192
000476	IF (T.EQ.16) CALL AXIS (-0.25,0.,33HMEAN PLASTIC INDEX (PER. DRY W	194
	1T.),33,10.,90.0,Y(I+1),Y(I+2),.14,.105,0)	196
000517	IF (T.EQ.17) CALL AXIS (-0.25,0.,28HMEAN GRAVEL (PERCENT SAMPLE),2	198
	18,10.,90.0,Y(I+1),Y(I+2),.14,.105,0)	200
000540	IF (T.EQ.18) CALL AXIS (-0.25,0.,26HMEAN SAND (PERCENT SAMPLE),26,	202
	110.,90.0,Y(I+1),Y(I+2),.14,.105,0)	204
000561	IF (T.EQ.19) CALL AXIS (-0.25,0.,26HMEAN SILT (PERCENT SAMPLE),26,	206
	110.,90.0,Y(I+1),Y(I+2),.14,.105,0)	208
000602	IF (T.EQ.20) CALL AXIS (-0.25,0.,26HMEAN CLAY (PERCENT SAMPLE),26,	210
	110.,90.0,Y(I+1),Y(I+2),.14,.105,0)	212
000623	IF (T.EQ.21) CALL AXIS (-0.25,0.,38HMEAN PASSING 200 MESH (PERCENT	214
	1 SAMPLE),38,10.,90.0,Y(I+1),Y(I+2),.14,.105,0)	216
000644	IF (T.EQ.22) CALL AXIS (-0.25,0.,41HMEAN HORZ. OR VERT. PERMEABILI	218
	1TY (CM/SEC),41,10.,90.0,Y(I+1),Y(I+2),.14,.105,1)	220
000665	IF (T.EQ.23) CALL AXIS (-0.25,0.,44HMEAN VOID RATIO (VOLUME VOIDS/	222
	1VOLUME SOLIDS),44,10.,90.0,Y(I+1),Y(I+2),.14,.105,1)	224
	C LABEL X-AXIS	226
	C	228
	C	230
000706	CALL AXIS (0.,0.,31HDEPTH BELOW LAND SURFACE (FEET),-31,09.,0.,5.,	232

```

C          PLOT SYMBOL                                294
C
001014      CALL PLT (XPAGE,YPAGE,3)                  296
001017      ICODE=-2                                  298
001020      150 CONTINUE                               300
001024      CALL PLT (0.,0.,999)                      302
C
C          CHECK FOR SYSTEM DEPENDENT PAPER ALLOCATION 304
C
001027      IF (T.EQ.7.OR.T.EQ.14.OR.T.EQ.21) CALL ENDPLT 306
001043      IF (T.EQ.7.OR.T.EQ.14.OR.T.EQ.21) CALL BGNPLT 308
001057      160 CONTINUE                               310
001062      CALL ENDPLT                               312
001063      RETURN                                    314
C
001065      170 FORMAT (* XPAGE) = *F9.5* YPAGE) = *F9.5* R(*I2*) = *A3) 316
001065      END                                       318
                                                    320
                                                    322
                                                    324
                                                    326-

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PROGRAM LENGTH INCLUDING I/O BUFFERS  
014030

FUNCTION ASSIGNMENTS

STATEMENT ASSIGNMENTS

120	=	000053	130	=	000070	140	=	000072	150	=	001021
160	=	001060	170	=	001413						

		15.,.14.,.105,0)	234
	C		236
	C	DRAW BORDER	238
	C		240
000721		CALL PLT (-0.25,10.,.3)	242
000724		CALL PLT (09.25,10.,.2)	244
000727		CALL PLT (09.25,0.,.2)	246
000732		CALL PLT (-0.25,0.,.2)	248
000735		DO 150 U=1,9	250
000737		ICODE=-1	252
000740		DO 150 D=1,10	254
000741		YA(1)=MEAN(U,D,T)	256
000746		IF (YA(1).EQ.0.) GO TO 150	258
000750		YA(2)=Y(I+1)	260
000751		YA(3)=Y(I+2)	262
000752		XA=D*5	264
	C		266
	C	DETERMINE PLOTTER COORDINATES	268
	C		270
000755		XPAGE=(XA-5.)/5.	272
000757		YPAGE=(YA(1)-YA(2))/YA(3)	274
	C		276
	C	DRAW LINE TO NEXT POINT	278
	C		280
000763		CALL SYMBOL (XPAGE,YPAGE,.04,9,0.,ICODE)	282
000767		XPAGE1=XPAGE-.15	284
000771		YPAGE1=YPAGE-.05	286
000773		PRINT 170, XPAGE1,YPAGE1,U,R(U)	288
001007		CALL SYMBOL (XPAGE1,YPAGE1,.1,R(U),0.,.3)	290
	C		292



BLOCK NAMES AND LENGTHS

VARIABLE ASSIGNMENTS

D	-	007741	I	-	007755	ICODE	-	007757	K	-	007756
MEAN	-	002020	NOTEST	-	007754	R	-	007743	T	-	007742
U	-	007740	XA	-	002014	XPAGE	-	007760	XPAGE1	-	007762
Y	-	001432	YA	-	002015	YPAGE	-	007761	YPAGE1	-	007763

START OF CONSTANTS

001070

START OF TEMPORARIES

001422

START OF INDIRECTS

001430

UNUSED COMPILER SPACE

004100